

DEPARTMENTAL POLICY

the cost of acquiring permanent and construction easements. Associated costs include engineering, legal and fiscal services. The CFC does not include the cost of constructing water distribution or wastewater collection systems.

Development:

Development has the meaning given it in F.S. 380.04.

Developer:

The owner of a parcel of land within the County's water and wastewater consolidated system service area and is desirous of purchasing water and wastewater treatment service from the County.

Equivalent Residential Connections (ERCs):

A factor used to convert a given annual daily flow (AADF) to the equivalent number of units required for connection to the County system. For residential purposes, all single family units, shall constitute one ERC. For non-residential use one ERC shall equal two hundred and fifty (250) gallons per day (AADF).

PROCEDURE

Developer shall submit a request for reservation of capacity in the form requested by Martin County Utilities and Solid Waste Department. The reservation of capacity request must include the calculation of ERCs needed to adequately supply the proposed existing Development with water and/or wastewater service. The Utilities and Solid Waste Department Staff shall review and either approve the request or submit comments to the developer concerning the request and/or calculations.

Once the Technical Services Administrator or his designee has approved the calculation, the developer will be able to reserve water and wastewater capacity either through execution of a water and wastewater service agreement or payment of Capital Facility Charges (CFCs).

Once the reservation of capacity has been determined by the calculation of ERC's and the execution of the water and wastewater service agreement, the reservation of capacity will be recorded in the official records as part of the water and wastewater service agreement. The Developer, property owner(s) or subsequent assignees to the water and wastewater service agreement shall not recalculate the reservaiton of capacity (ERC's).

John Polley, Utilities & Solid Waste Director

Suppression History:

Created 12/20/05

Revised 7/06

Revised 1/26/12

Florida Administrative Code – System Size Determinations

64E-6.008 System Size Determinations.

(1) Minimum design flows for systems serving any structure, building or group of buildings shall be based on the estimated daily sewage flow as determined from Table I or the following:

(a) The DOH county health department shall accept, for other than residences and food operations, metered water use data in lieu of the estimated sewage flows set forth in Table I. For metered flow consideration, the applicant shall provide authenticated monthly water use data documenting water consumption for the most recent 12 month period for at least six similar establishments. Similar establishments are those like size operations engaged in the same type of business or service, which are located in the same type of geographic environment, and which have approximately the same operating hours. Metered flow values will not be considered to be a reliable indicator of typical water use where one or more of the establishments utilized in the sample has exceeded the monthly flow average for all six establishments by more than 25 percent or where the different establishments demonstrate wide variations in monthly flow totals. When metered flow data is accepted in lieu of estimated flows found in Table I, the highest flow which occurred in any month for any of the six similar establishments shall be used for system sizing purposes. Except for food operations which exceed domestic sewage waste quality parameters as defined in subsection 64E-6.002(15), F.A.C., where an existing establishment which has been in continuous operation for the previous 24 months seeks to utilize its own metered flows, the applicant shall provide authenticated monthly water use data documenting water consumption for the most recent 24 month period. The highest monthly metered flow value for an existing establishment shall be used for system sizing purposes.

(b) When onsite systems use multiple strategies to reduce the total estimated sewage flow or the drainfield size, only one reduction method shall be credited.

TABLE I
For System Design
ESTIMATED SEWAGE FLOWS

TYPE OF ESTABLISHMENT COMMERCIAL:	GALLONS PER DAY
Airports, bus terminals, train stations, port & dock facilities, Bathroom waste only	
(a) Per passenger	4
(b) Add per employee per 8 hour shift	15
Barber & beauty shops per service chair	75
Bowling alley bathroom waste only per lane	50
Country club	
(a) Per resident	100
(b) Add per member or patron	25
(c) Add per employee per 8 hour shift	15
Doctor and Dentist offices	
(a) Per practitioner	250
(b) Add per employee per 8 hour shift	15
Factories, exclusive of industrial wastes gallons per employee per 8 hour shift	
(a) No showers provided	15
(b) Showers provided	25
Flea Market open 3 or less days per week	
(a) Per non-food service vendor space	15
(b) Add per food service establishment using single service articles only per 100 Square feet of floor space	50
(c) Per limited food service establishment	25
(d) For flea markets open more than 3 days per week estimated flows shall be doubled	
Food operations	
(a) Restaurant operating 16 hours or less per day per seat	40
(b) Restaurant operating more than 16 hours per day per seat	60
(c) Restaurant using single service articles only and operating 16 hours or less per day per seat	20
(d) Restaurant using single service articles only and operating more than 16 hours per day per seat	35
(e) Bar and cocktail lounge per seat	20

Florida Administrative Code – System Size Determinations

add per pool table or video game	15
(f) Drive-in restaurant per car space	50
(g) Carry out only, including caterers	
1. Per 100 square feet of floor space	50
2. Add per employee per 8 hour shift	15
(h) Institutions per meal	5
(i) Food Outlets excluding deli's, bakery, or meat department per 100 square feet of floor space	10
1. Add for deli per 100 square feet of deli floor space	40
2. Add for bakery per 100 square feet of bakery floor space	40
3. Add for meat department per 100 square feet of meat department floor space	75
4. Add per water closet	200
Hotels & motels	
(a) Regular per room	100
(b) Resort hotels, camps, cottages per room	200
(c) Add for establishments with self service laundry facilities per machine	750
Mobile Home Park	
(a) Per single wide mobile home space, less than 4 single wide spaces connected to a shared onsite system	250
(b) Per single wide mobile home space, 4 or more single wide spaces are connected to a shared onsite system	225
(c) Per double wide mobile home space, less than 4 double wide mobile home spaces connected to a shared onsite system	300
(d) Per double wide mobile home space, 4 or more double wide mobile home spaces connected to a shared onsite system	275
Office building	
per employee per 8 hour shift, or	15
per 100 square feet of floor space, whichever is greater	15
Transient Recreational Vehicle Park	
(a) Recreational vehicle space for overnight stay, without water and sewer hookup per vehicle space	50
(b) Recreational vehicle space for overnight stay, with water and sewer hookup per vehicle space	75
Service stations per water closet	
(a) Open 16 hours per day or less	250
(b) Open more than 16 hours per day	325
Shopping centers without food or laundry per square foot of floor space	0.1
Stadiums, race tracks, ball parks per seat	4
Stores per bathroom	200
Swimming and bathing facilities, public	10
per person	
Theatres and Auditoriums, per seat	4
Veterinary Clinic	
(a) Per practitioner	250
(b) Add per employee per 8 hour shift	15
(c) Add per kennel, stall or cage	20
Warehouse	
(a) Add per employee per 8 hour shift	15
(b) Add per loading bay	100
(c) Self-storage, per unit (up to 200 units)	1
add 1 gallon for each 2 units or fraction thereof, for over 200 units, and shall be in addition to employees, offices or living quarters flow rates.	

Florida Administrative Code – System Size Determinations

INSTITUTIONAL:

Churches per seat which includes kitchen wastewater flows unless meals prepared on a routine basis	3
If meals served on a regular basis add per meal prepared	5
Hospitals per bed which does not include kitchen wastewater flows add per meal prepared	200
Nursing, rest homes, adult congregate living facilities per bed which does not include kitchen wastewater flows add per meal prepared	100
Parks, public picnic	
(a) With toilets only per person	4
(b) With bathhouse, showers & toilets per person	10
Public institutions other than schools and hospitals per person which does not include kitchen wastewater flows add per meal prepared	100
Schools per student	
(a) Day-type	10
(b) Add for showers	4
(c) Add for cafeteria	4
(d) Add for day school workers	15
(e) Boarding-type	75
Work/construction camps, semi-permanent per worker	50

RESIDENTIAL:

Residences	
(a) Single or multiple family per dwelling Unit	
1 Bedroom with 750 sq. ft. or less of building area	100
2 Bedrooms with 751-1,200 sq. ft. of building area	200
3 Bedrooms with 1,201-2,250 sq. ft. of building area	300
4 Bedrooms with 2,251-3,300 sq. ft. of building area	400
For each additional bedroom or each additional 750 square feet of building area or fraction thereof in a dwelling unit, system sizing shall be increased by 60 gallons per dwelling unit.	
(b) Other per occupant	50

Footnotes to Table I:

- For food operations, kitchen wastewater flows shall normally be calculated as 66 percent of the total establishment wastewater flow.
- Systems serving high volume establishments, such as restaurants, convenience stores and service stations located near interstate type highways and similar high-traffic areas, require special sizing consideration due to expected above average sewage volume. Minimum estimated flows for these facilities shall be 3.0 times the volumes determined from the Table I figures.
- For residences, the volume of wastewater shall be calculated as 50 percent blackwater and 50 percent graywater.
- Where the number of bedrooms indicated on the floor plan and the corresponding building area of a dwelling unit in Table I do not coincide, the criteria which will result in the greatest estimated sewage flow shall apply.
- Convenience store estimated sewage flows shall be determined by adding flows for food outlets and service stations as appropriate to the products and services offered.
- Estimated flows for residential systems assumes a maximum occupancy of two persons per bedroom. Where residential care facilities will house more than two persons in any bedroom, estimated flows shall be increased by 50 gallons per each additional occupant.
- (2) Minimum effective septic tank capacity and total dosing tank capacity shall be determined from Table II. However, where

Florida Administrative Code – System Size Determinations

multiple family dwelling units are jointly connected to a septic tank system, minimum effective septic tank capacities specified in the table shall be increased 75 gallons for each dwelling unit connected to the system. With the exception noted in paragraph 64E-6.013(2)(a), F.A.C., all septic tanks shall be multiple chambered or shall be placed in series to achieve the required effective capacity. The use of an approved outlet filter device shall be required. Outlet filters shall be installed within or following the last septic tank or septic tank compartment before distribution to the drainfield. The outlet filter device requirement includes blackwater tanks, but does not include graywater tanks or grease interceptors or laundry tanks. Outlet filter devices shall be placed to allow accessibility for routine maintenance. Utilization and sizing of outlet filter devices shall be in accordance with the manufacturers' recommendations. The approved outlet filter device shall be installed in accordance with the manufacturers' recommendations. The Bureau of Onsite Sewage Programs shall approve outlet filter devices per the department's Policy on Approval Standards For Onsite Sewage Treatment And Disposal Systems Outlet Filter Devices, November 2008, which is herein incorporated by reference.

TABLE II
SEPTIC TANK AND PUMP TANK CAPACITY

AVERAGE SEWAGE FLOW GALLONS/DAY	SEPTIC TANK	PUMP TANK	
	MINIMUM EFFECTIVE CAPACITY GALLONS	MINIMUM TOTAL CAPACITY GALLONS	
		Residential	Commercial
0-200	900	150	225
201-300	900	225	375
301-400	1,050	300	450
401-500	1,200	375	600
501-600	1,350	450	600
601-700	1,500	525	750
701-800	1,650	600	900
801-1,000	1,900	750	1,050
1,001-1,250	2,200	900	1,200
1,251-1,750	2,700	1,350	1,900
1,751-2,500	3,200	1,650	2,700
2,501-3,000	3,700	1,900	3,000
3,001-3,500	4,300	2,200	3,000
3,501-4,000	4,800	2,700	3,000
4,001-4,500	5,300	2,700	3,000
4,501-5,000	5,800	3,000	3,000

(3) Where a separate graywater tank and drainfield system is used, the minimum effective capacity of the graywater tank shall be 250 gallons with such system receiving not more than 75 gallons of flow per day. For graywater systems receiving flows greater than 75 gallons per day, minimum effective tank capacity shall be based on the average daily sewage flow plus 200 gallons for sludge storage. Design requirements for graywater tanks are described in subsection 64E-6.013(2), F.A.C. Where separate graywater and blackwater systems are utilized, the size of the blackwater system can be reduced, but in no case shall the blackwater system be reduced by more than 25 percent. However, the minimum capacity for septic tanks disposing of blackwater shall be 900 gallons.

(4) Where building codes allow separation of discharge pipes of the residence to separate stubouts and where lot sizes and setbacks allow system construction, the applicant may request a separate laundry waste tank and drainfield system. Where an aerobic treatment unit is used, all blackwater, graywater and laundry waste flows shall be consolidated and treated by the aerobic treatment unit. Where a residential laundry waste tank and drainfield system is used:

(a) The minimum laundry waste trench drainfield absorption area for slightly limited soil shall be 75 square feet for a one or two bedroom residence with an additional 25 square feet for each additional bedroom. If an absorption bed drainfield is used the minimum drainfield area shall be 100 square feet with an additional 50 square feet for each additional bedroom over two bedrooms. The DOH county health department shall require additional drainfield area based on moderately limited soils and other site specific conditions, which shall not exceed twice the required amount of drainfield for a slightly limited soil.

(b) The laundry waste interceptor shall meet requirements of subsections 64E-6.013(2) and (8), F.A.C.

(c) The drainfield absorption area serving the remaining wastewater fixtures in the residence shall be reduced by 25 percent.

(5) The minimum absorption area for standard subsurface drainfield systems, graywater drainfield systems, and filled systems



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Colorado Secretary of State
Date and Time: 09/21/2015 05:39 PM
ID Number: 20151603485
Document number: 20151603485
Amount Paid: \$50.00

ABOVE SPACE FOR OFFICE USE ONLY

Articles of Organization

filed pursuant to § 7-80-203 and § 7-80-204 of the Colorado Revised Statutes (C.R.S.)

1. The domestic entity name of the limited liability company is

Advantage Development Group LLC

(The name of a limited liability company must contain the term or abbreviation "limited liability company", "ltd. liability company", "limited liability co.", "ltd. liability co.", "limited", "l.l.c.", "llc", or "ltd.". See §7-90-601, C.R.S.)

(Caution: The use of certain terms or abbreviations are restricted by law. Read instructions for more information.)

2. The principal office address of the limited liability company's initial principal office is

Street address

34628 Lyttle Dowdle Dr

(Street number and name)

Golden

CO

80403

(City)

(State)

(ZIP/Postal Code)

CO

United States

(Province – if applicable)

(Country)

Mailing address

(leave blank if same as street address)

P.O. Box 7324

(Street number and name or Post Office Box information)

Golden

CO

80403

(City)

(State)

(ZIP/Postal Code)

CO

United States

(Province – if applicable)

(Country)

3. The registered agent name and registered agent address of the limited liability company's initial registered agent are

Name

(if an individual)

(Last)

(First)

(Middle)

(Suffix)

or

(if an entity)

Advantage Advisors, LLC

(Caution: Do not provide both an individual and an entity name.)

Street address

34628 Lyttle Dowdle

(Street number and name)

Golden

CO

80403

(City)

(State)

(ZIP Code)

Mailing address

(leave blank if same as street address)

P.O. Box 7324

(Street number and name or Post Office Box information)

Golden CO 80403
(City) (State) (ZIP Code)

(The following statement is adopted by marking the box.)

☒ The person appointed as registered agent has consented to being so appointed.

4. The true name and mailing address of the person forming the limited liability company are

Name
(if an individual)

(Last) (First) (Middle) (Suffix)
or

(if an entity) Advantage Advisors, LLC
(Caution: Do not provide both an individual and an entity name.)

Mailing address P.O. Box 7324
(Street number and name or Post Office Box information)

Golden CO 80403
(City) (State) (ZIP/Postal Code)
CO United States
(Province – if applicable) (Country)

(If the following statement applies, adopt the statement by marking the box and include an attachment.)

☐ The limited liability company has one or more additional persons forming the limited liability company and the name and mailing address of each such person are stated in an attachment.

5. The management of the limited liability company is vested in

(Mark the applicable box.)

☐ one or more managers.

or

☒ the members.

6. (The following statement is adopted by marking the box.)

☒ There is at least one member of the limited liability company.

7. (If the following statement applies, adopt the statement by marking the box and include an attachment.)

☐ This document contains additional information as provided by law.

8. (Caution: Leave blank if the document does not have a delayed effective date. Stating a delayed effective date has significant legal consequences. Read instructions before entering a date.)

(If the following statement applies, adopt the statement by entering a date and, if applicable, time using the required format.)

The delayed effective date and, if applicable, time of this document is/are _____
(mm/dd/yyyy hour:minute am/pm)

Notice:

Causing this document to be delivered to the Secretary of State for filing shall constitute the affirmation or acknowledgment of each individual causing such delivery, under penalties of perjury, that the document is the individual's act and deed, or that the individual in good faith believes the document is the act and deed of the person on whose behalf the individual is causing the document to be delivered for filing, taken in conformity with the requirements of part 3 of article 90 of title 7, C.R.S., the constituent documents, and the organic statutes, and that the individual in good faith believes the facts stated in the document are true and the document complies with the requirements of that Part, the constituent documents, and the organic statutes.

This perjury notice applies to each individual who causes this document to be delivered to the Secretary of State, whether or not such individual is named in the document as one who has caused it to be delivered.

9. The true name and mailing address of the individual causing the document to be delivered for filing are

Kinder		Jeffrey	
	(Last)		(First)
34628 Lyttle Dowdle Dr			
(Street number and name or Post Office Box information)			
Golden		CO	80403
	(City)	(State)	(ZIP/Postal Code)
CO		United States	
(Province – if applicable)		(Country)	

(If the following statement applies, adopt the statement by marking the box and include an attachment.)

- ☐ This document contains the true name and mailing address of one or more additional individuals causing the document to be delivered for filing.

Disclaimer:

This form/cover sheet, and any related instructions, are not intended to provide legal, business or tax advice, and are furnished without representation or warranty. While this form/cover sheet is believed to satisfy minimum legal requirements as of its revision date, compliance with applicable law, as the same may be amended from time to time, remains the responsibility of the user of this form/cover sheet. Questions should be addressed to the user's legal, business or tax advisor(s).

B BETSY LINDSAY, INC.

7907 S.W. JACK JAMES DRIVE STUART, FLORIDA 34987
 (772) 288-5753 (772) 288-5833 FAX
 LICENSED BUSINESS NO. 6652

ALTA BOUNDARY SURVEY

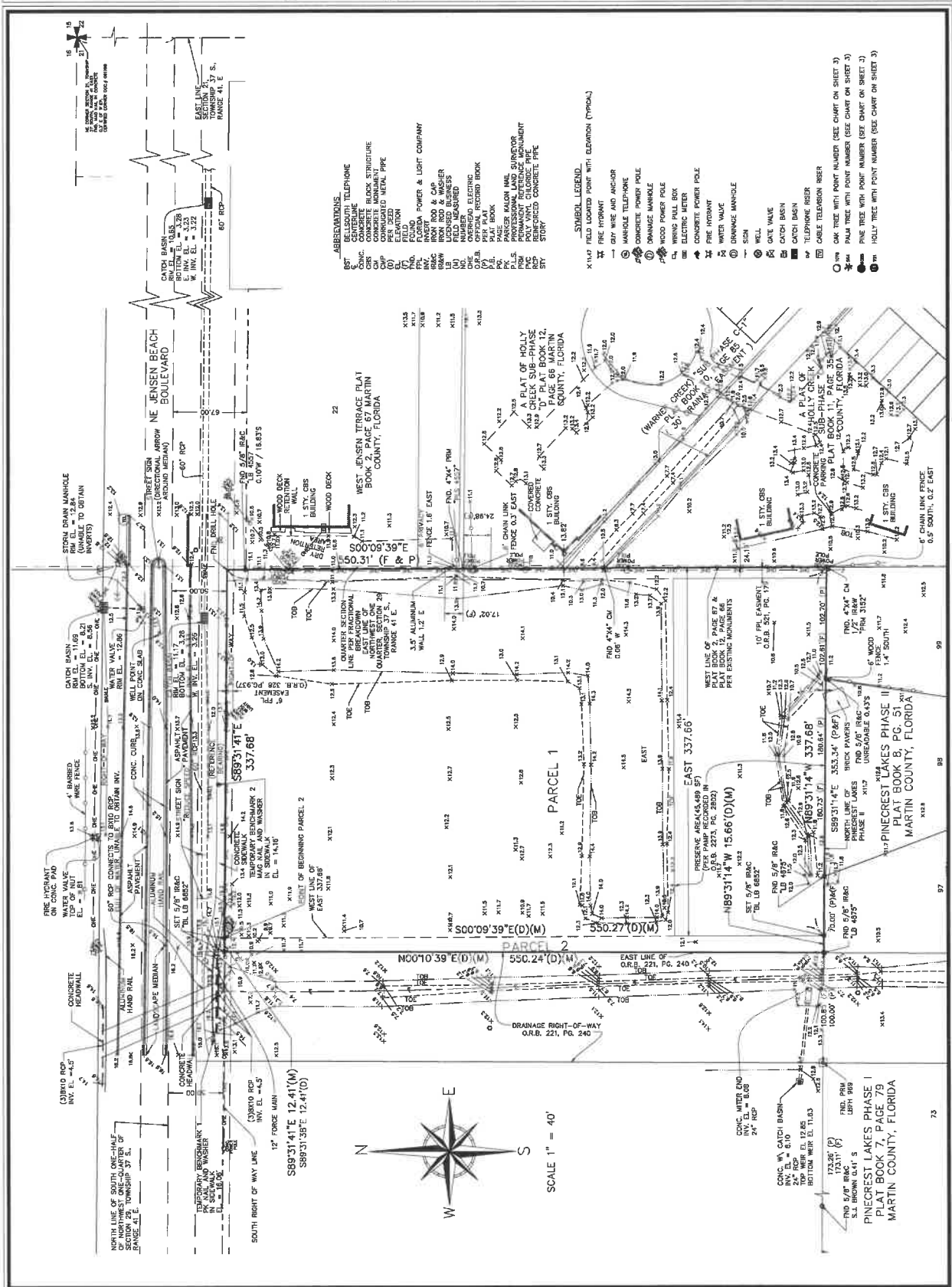
A PORTION OF THE S. 1/2 OF THE NW 1/4,
 SECTION 21, TOWNSHIP 37 S., RANGE 41 E.,
 JENSEN BEACH, MARTIN COUNTY, FLORIDA

DATE: 05/07/10
 SCALE: 1" = 40'

FIELD BY: JORDEN BECHTOLD
 DRAWN BY: GUY P. LEE
 CHECKED BY: E.A.L.

DATE: 05/07/10
 SCALE: 1" = 40'

FIELD BY: JORDEN BECHTOLD
 DRAWN BY: GUY P. LEE
 CHECKED BY: E.A.L.





Engineering & Planning, Inc.

1172 SW 30th Street • Suite 500 • Palm City • Florida • 34990

(772) 286-8030 • www.MackenzieEngineeringInc.com

August 31, 2018

Mr. Paul Schilling
Martin County Growth Management
2401 SE Monterey Road
Stuart FL, 34996

Re: Advantage Development Group
Advantage Self Storage – Final Site Plan
Utilities-Related Calculations

Dear Mr. Schilling:



Digitally signed
by Shaun G
MacKenzie
Date:
2018.09.07
09:11:29 -04'00'

Shaun G. MacKenzie P.E.
FL License No. 61751

INTRODUCTION

This document provides a summary of the proposed utilities-related calculation associated with the Advantage Self Storage – Final Site Plan project ("Project") located in Jensen Beach, Florida. The site will consist of a lift station, irrigation system, fire sprinkler system and a fire hydrant.

LIFT STATION

A 4' x 14' with 3 HP grinder pump is proposed for the site to connect to the Martin County force main adjacent to the site. The lift station capacity is designed to have a minimum capacity of 600 gallons per day (GPD) based on the Martin County Water & Wastewater Systems Calculation (using the Florida Administrative Code). The existing force main has recorded pressure of 10 psi. We propose to use 30 psi at buildout in order to provide a conservative analysis. The water/wastewater use is 250 gallons per day divided by an average consumption of water at a rate of 250 GPD (considered equal to 1 ERC). As a result, 1 ERC is needed (see attached ERC Reduction Analysis).

IRRIGATION

A two-inch well is proposed for irrigation purposes. Please see attached summary of groundwater (well) facilities. The site is adjacent to an 8" re-use (reclaimed) water line. However, per Martin County Utility, reclaimed water is not available for this project.

GREASE INTERCEPTOR SIZING

Not Applicable.

FIRE FLOW (NON-RESIDENTIAL)

The fire hydrant will be constructed per Martin County design requirements. The Fire Flow calculation is based on the design criteria for

- Ordinary hazard group 2
 - Density of .20 GPM per SF for all areas over 1500 SF; and
 - 130 SF maximum spacing per sprinkler in the storage areas.
- Light Hazard
 - Density of .10 GPM per SF for all over 1500 SF; and
 - 225 SF maximum spacing per sprinkler in the office areas.

The fire sprinklers will be fed by a 6" fire line with a single 6" backflow preventer.

ATTACHMENTS:

1. Existing Utilities
- 2a. ERC Reduction Analysis
- 2b. Lift Station
 - Pressure Test on Existing Force Main
 - Lift Station Summary
 - MOPS Engineering Report
 - Lift Station Design
3. Irrigation
 - Summary of Groundwater (Well) Facility
4. Fire Flow
 - Existing Fire Hydrant Flow
 - Fire Flow Calculation

ATTACHMENT 1: EXISTING UTILITIES

520 NE Jensen Beach Blvd

Existing 6" water main

Existing 16" water main

NE Jensen Beach Blvd

Fire Hydrant 55

Fire Hydrant 63

0

300 Feet

Date: 11/1/2017
This Geographic Information System Map Product received from Martin County (COUNTY) in fulfillment of a public records request is provided "as is" without warranty of any kind, and the COUNTY expressly disclaims all express and implied warranties, including but not limited to the implied warranties of merchantability and fitness for a particular purpose. The COUNTY does not warrant, guarantee, or make any representation regarding the use of the results of the use of the information provided to you by the COUNTY, in terms of correctness, accuracy, reliability, timeliness or otherwise. The entire risk as to the results and performance of any information obtained from the COUNTY is entirely assumed by the recipient.



Author: Martin County Utilities
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ATTACHMENT 2a: ERC REDUCTION ANALYSIS



Engineering & Planning, Inc.

1172 SW 30th Street • Suite 500 • Palm City • Florida • 34990

(772) 286-8030 • www.mackenzieengineeringinc.com

August 29, 2018

Leo Repetti, PE
Martin County Utilities & Solid Waste
Martin County Growth Management
2401 SE Monterey Road
Stuart, FL 34996
Phone: 772-320-3065
Email: lrepetti@martin.fl.us

Re: Advantage Development Group
Advantage Self Storage – Revised Major Final Site Plan
ERC Reduction Request

MacKenzie Engineering & Planning, Inc. was retained to determine an estimated water usage for a Mini-Self Storage. The applicant proposes to construct a 92,700 SF storage facility at 528 NE Jensen Beach Blvd, Jensen Beach, Florida.

An analysis was performed of three (3) similar Self Storage Facilities owned by Advantage Development Group to determine an average water usage/ERC for such use. The projected use will have 1 restroom and no more than 2 employee's onsite. Based on our analysis, the development is projected to use 194.7 GPD (0.78 ERC's). Please see attached analysis and supporting documentation.

If you have any questions, please do not hesitate to contact Shaun Mackenzie at (772) - 834-8909 or shaun@mackenzieengineeringinc.com.

Sincerely,

A handwritten signature in cursive script that reads 'Shaun MacKenzie'.

Shaun G. MacKenzie, P.E.
Transportation Engineer
Florida Registration Number 61751
Engineering Business Number 29013

Water Usage Comparison for a Mini-Self Storage Facility											
(A)	(B)	(C)	(D)	(E)	(F)	(G)	(H)	(I)	(J)	(K)	(L)
Property	Site Address/Location	Water Usage (Gal.) *	Billing Period	# of Days	Avg. Gal. Per Day (GPD)	Bldg. Size (KSF) **	Avg. GPD KSF	Avg. Water Usage (GPD/KSF)	Proposed Site Size (KSF)	Proposed GPD	Proposed ERC
Mixed AADC 021 - Silva Corp LLC	97 Rantoul Street, Beverly MA	546	10/24/17 - 11/24/17	31	18	29.564	0.60				
Advantage Storage	10 Council Drive, Woodshoro, MD	23,069	09/25/17 - 12/27/17	93	248	117.555	2.11				
Advantage US Holdings 111 LLC	2938 Walden Ave Depew, NY	44,000	11/05/17 - 02/04/18	91	484	134.524	3.59	2.10	92.7	194.7	0.78

* Water Bills of Self Storage Facilities owned by Advantage Development Group. Mixed AADC 021 site provides their water usage per cubic feet therefore, 73 CF * 7.48 gal/CF = 546 gallons

** Estimate based on aerial photography

(F) = (E) / (C)

(H) = (F) / (G)

(I) = SUM(H) / 3

(K) = (I) * (J)

(L) = (K) / 250 gallons per MC Code (1 ERC = 250 GPD)

City of Salem
Office of the Tax Collector
93 Washington Street
Salem, MA 01970
000005 0000905



City of Salem Water & Sewer Bill

You can pay your water & sewer bill
on-line at www.salem.com

Account #	Bill #
M82001	1108244

Failure to receive this bill does not relieve the customer of the responsibility to pay.
Amounts not received on or before the due date are subject to interest charges.

MIXED AADC 021
SILVACOR, INC
736 MAIN AVE STE 7
DURANGO CO 81301-5479



Questions regarding billing, please contact (978) 619-5674
Questions regarding payments, please contact (978) 619-5621

Water & Sewer FY 2018
Customer Copy

Account #	Bill #	Parcel ID	Bill Date	Due Date	Past Due			
M82001	1108244	250492	12/07/2017	12/28/2017	\$0.00			
Service Location					Interest on Past Due			
4 JEFFERSON AVENUE					\$0.00			
Service Type	Previous Read Date	Current Read Date	Read Code	Previous Reading	Current Reading	Usage (c.f.)	Charges	Current Total
MO W 2.0 NS	10/24/2017	11/24/2017	A	3063	3136	73	\$80.80	\$87.52
MO S 2.0 NS			A			73	\$6.72	Total Amount Due
								\$87.52
								Discounted amount due if paid on or before 12/13/2017
								\$87.52
			REPLACED METER USAGE			0		

READING / USAGE CODES		DISTRICTS' ISSUE DATES
A - ACTUAL	O - ACTUAL READ, MANUAL ENTRY	DISTRICT A: BILLS ISSUED OCTOBER, JANUARY, APRIL, JULY
E - SYSTEM ESTIMATE	T - TROUBLESHOOT	DISTRICT B: BILLS ISSUED AUGUST, NOVEMBER, FEBRUARY, MAY
P - IN-HOUSE ESTIMATE	H - NON-RESPONSIVE - METER REPLACEMENT	DISTRICT C: BILLS ISSUED SEPTEMBER, DECEMBER, MARCH, JUNE

SEE REVERSE SIDE FOR IMPORTANT INFORMATION



SILVACOR, INC
736 MAIN AVE STE 7
DURANGO CO 81301-5479-5479

Account #	Bill #	Parcel ID	Past Due	\$0.00
M82001	1108244	250492	Interest on Past Due	\$0.00
Service Location				
4 JEFFERSON AVENUE			Total Amount Due →→	\$87.52
Bill Date: 12/07/2017	DISCOUNT PAYMENT MUST BE RECEIVED IN COLLECTOR'S OFFICE ON OR BEFORE: 12/13/2017		LESS DISCOUNT AMOUNT:	\$0.00
REMIT THIS SECTION WITH CHECK PAYABLE TO: CITY OF SALEM			Total Discounted Amount Due:	\$87.52

MAIL TO:

City of Salem
P.O. Box 4125
Woburn, MA 01888-4125

☐ Check here if your address has changed and note on reverse side.

Total Enclosed: \$ 319

03726042018001108244300000087528

Water & Sewer FY 2018
Remit Copy

Corporation of Woodsboro
2 S. Third Street
PO Box 88
Woodsboro, MD 21798

Utility Bill

31062

ADVANTAGE STORAGE
PO BOX 663
ELGIN IL 60121-0663

Account Number	Service Address	Previous Balance	Payments/Credits
032254	10 COUNCIL DRIVE	\$313.31	\$313.31

Charge	From	To	Previous	Current	Type	Consumption	Amount
Water	09/25/17	12/27/17	637094	660163	Actual	23069	\$111.50
Flush Fee	09/29/17	12/27/17					\$7.50
Sewer	09/25/17	12/27/17					\$193.72
							\$312.72

HAPPY NEW YEAR!!!!

Christmas tree pick-up will be on January 10, 2018 and January 17, 2018.
Have trees at the curb by 6:00 AM.

Total Due: \$312.72

Spring Bulk Trash pick up will be on April 18, 2018.

If Paid After 1/31/2018 \$317.41

Yard waste pick up will begin April 4, 2018 and end November 28, 2018

DETACH AND RETURN THIS PORTION WITH YOUR PAYMENT

Account Number	Customer	Service Address	Total Due
032254	ADVANTAGE STORAGE	10 COUNCIL DRIVE	\$312.72



Total Amount Enclosed:

Bill Date:

12/27/2017

Due Date:

1/31/2018

QUARTERLY STATEMENT

Page 1 of 1

Account Number 24445033-6
 Billing Date 2/01/2018
 Billing Period 11/05/2017 TO 2/04/2018
 91 Days

Meter Number 31577778
 For Service at
 2938 WALDEN AVE DEPEW NY

Amount Due \$160.15
 Due Date Mar 8, 2018

Meter Read Information
 Present 1,046,000 ECWA Rding
 Previous 1,002,000 ECWA Rding
 Gallon Usage 44,000

SUMMARY
 Amount of Last Bill \$98.90
 Payments Received, Thank You! \$98.90
 Previous Balance Due \$0.00

Current Water Charges
 27,000 @ 3.17/thous \$85.59
 17,000 @ 3.23/thous \$54.91
 Other Charges
 Infrastructure Invest Charge \$19.65
 Total Current Charges \$160.15
 Previous Balance Due \$0.00
 Total Amount Due \$160.15

To avoid a 10% late charge on the current bill, payment must be received by Mar 18, 2018.

Contacting ECWA

Office Hours: M-F 8am to 5pm
 Office (716) 849-8444
 Fax (716) 849-8467
 Emergency Phone (716) 684-0900
www.ecwa.org

This account is enrolled in ECWA online under
 AFREDRICKSEN@ADVANTAGESTORAGE.COM

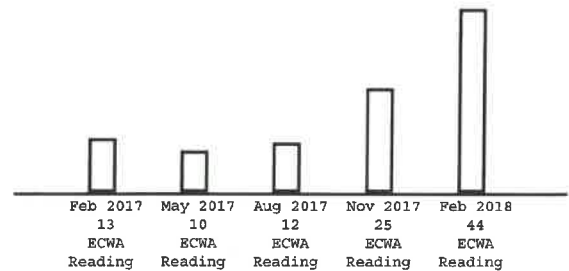
Messages

Your account is currently enrolled for automatic payments

If you change your bank or your account number at the bank, please be sure to update your on-line profile to ensure your payment is made on time and from the correct account

ECWA's 2016 Annual Water Quality Report is available on our website at www.ecwa.org

Quarterly Usage (in thousands)



OTHER QUARTERS HAD SIGNIFANTLY LOWER WATER USE THAN FEB 2018



Erie County Water Authority
 PO Box 5148
 Buffalo, NY 14240-5148

Phone (716) 849-8444 Office Hours: M-F 8am to 5pm



ACCOUNT NUMBER

24445033-6

Amount Due \$160.15

Due Date Mar 8, 2018

Amount

Enclosed \$

For Service at 2938 WALDEN AVE
 DEPEW NY

ADVANTAGE US HOLDINGS III LLC
 PO BOX 663
 ELGIN IL 60121

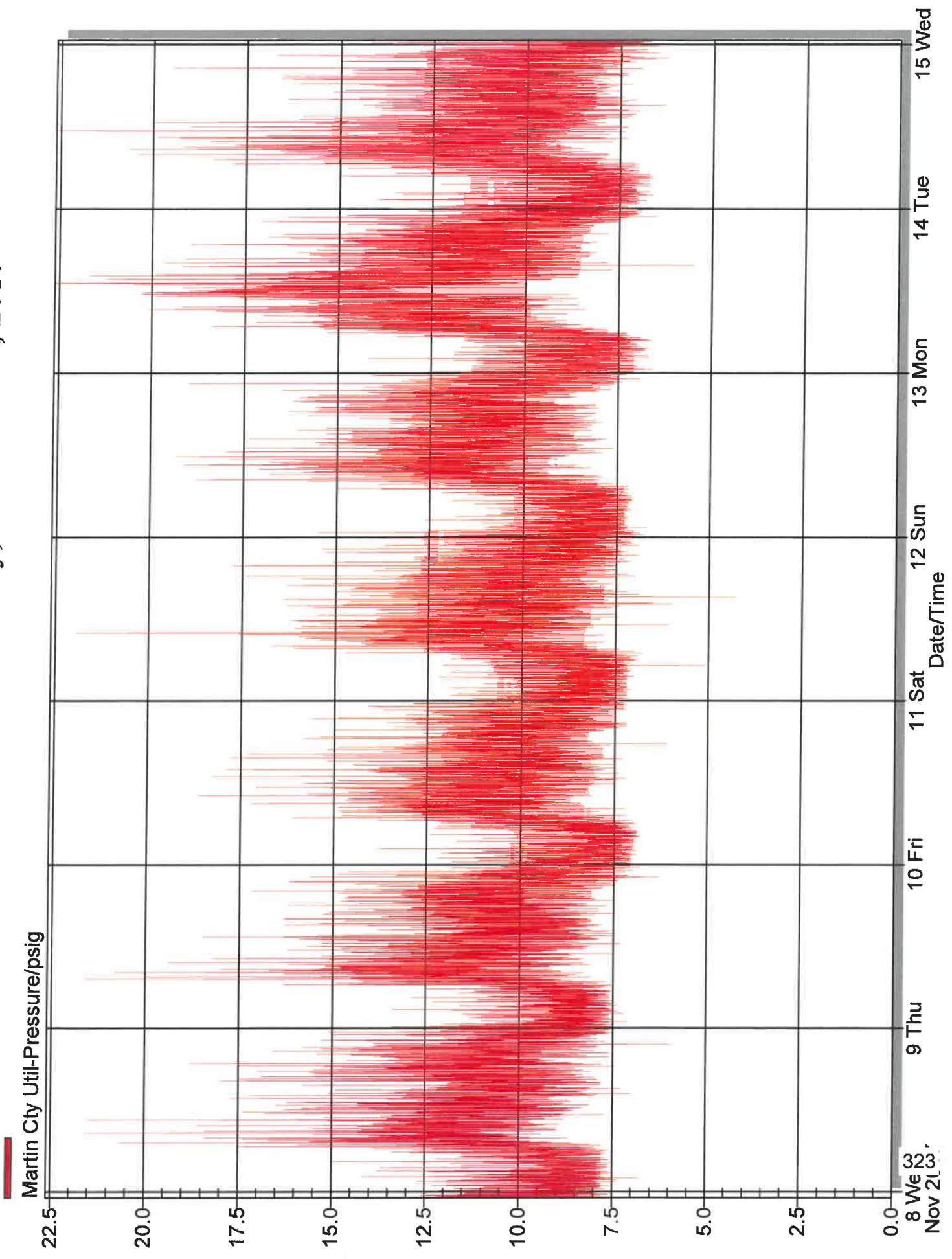
ERIE COUNTY WATER AUTHORITY
 PO BOX 5148
 BUFFALO NY 14240-5148

321

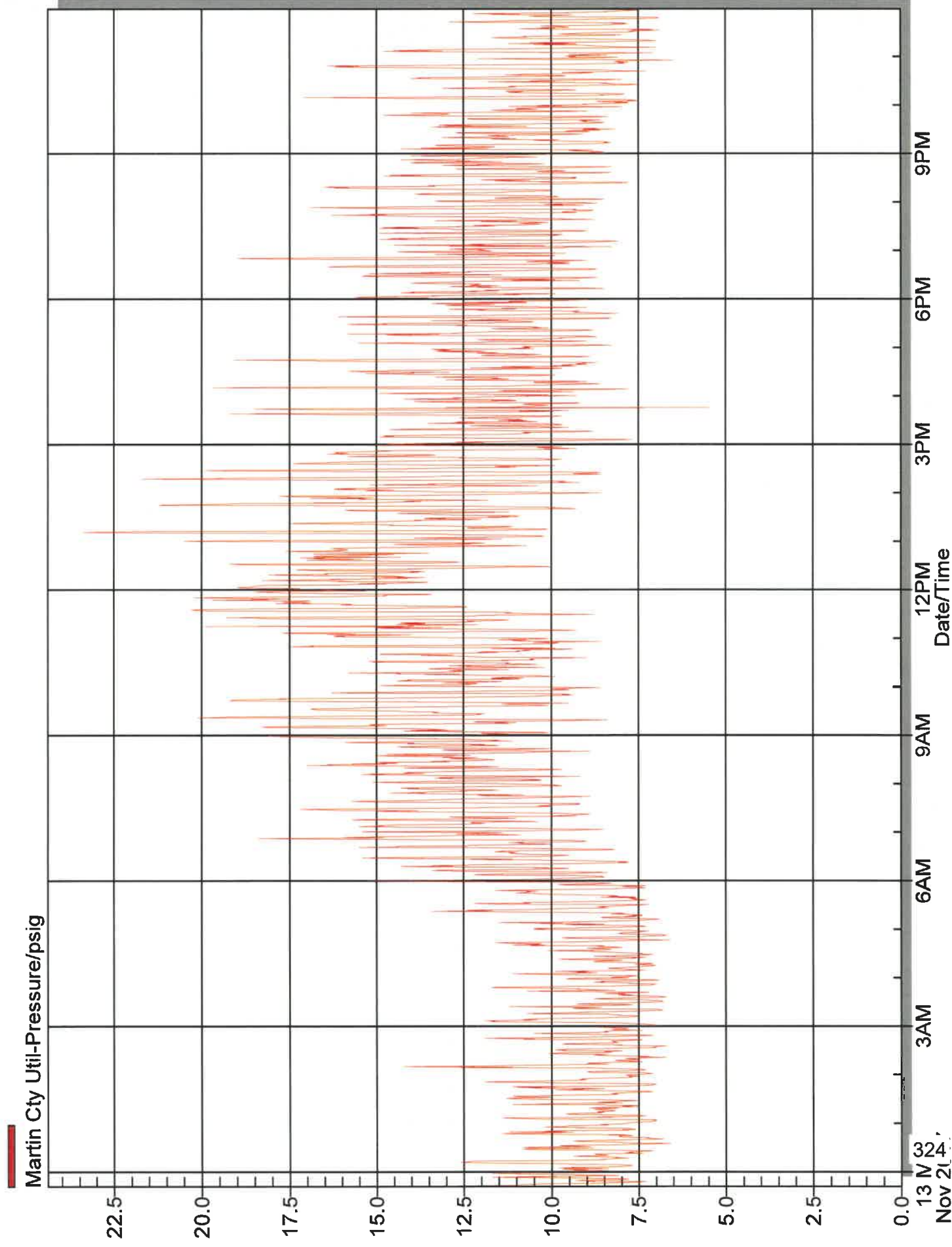
244450336000016015

ATTACHMENT 2b:
LIFT STATION
(PRESSURE TEST IN EXISTING
FORCE MAIN)

Downloaded Data - Wednesday, November 15, 2017



Downloaded Data - Wednesday, November 15, 2017



ATTACHMENT 2b:
LIFT STATION
(LIFT STATION SUMMARY)

PART II – PROJECT DOCUMENTATION

(1) Collection/Transmission System Permittee

Name _____ Title Project Engineer
 Company Name Martin County Utilities & Solid Waste
 Address 2378 SE Ocean Blvd.
 City Stuart State FL Zip 34996
 Telephone 772-320-3065 Fax _____ Email lrepetti@martin.fl.us

(2) General Project Information

Project Name _____
 Location: County Martin City Stuart Section 21 Township 37S Range 41E
 Project Description and Purpose (including pipe length, range of pipe diameter, total number of manholes, and total number of pump stations):
Advantage Development Group proposes a 92,700 SF self-storage facility with approximately 850 units. The site proposes +/- 108 LF, 4" PVC Gravity Sanitary Service, 0 manholes, and 1 pump station.
 Estimated date for: Start of construction January 2019 Completion of construction December 2020
 Connections to existing system or treatment plant Yes, connection to existing 12" FM

(3) Project Capacity

A = Type of Unit	B = Number of Units	C = Population Per Unit	D = Total Population (Columns B x C)	E = Per Capita Flow	F = Total Average Daily Flow (Columns D x E)	G = Peak hour flow
Single-Family Home						
Mobile Home						
Apartment						
Commercial, Institutional, or Industrial Facility*					250	89
Total					250	89

* Description of commercial, institutional, and industrial facilities and explanation of method used to estimate per capita flow for these facilities:

(4) Pump Station Data (attached additional sheets as necessary)

Location	Type	Estimated Flow to the Station (GPD)			Operating Conditions [GPM @ FT (TDH)]
		Maximum	Average	Minimum	
On-Site	Duplex Submersible	1,063	250	0	13 GPM @ 82' TDH
	Sewage Grinder				23 GPM @ 39' TDH
	Pump Station				

(5) Collection/Transmission System Design Information

- A. This information must be completed for all projects by the applicant's professional engineer, and if applicable, those professional engineers in other disciplines who assisted with the design of the project.

If this project has been designed to comply with the standards and criteria listed below, the engineer shall initial in ink before the standards or criteria. If any of the standards or criteria do not apply to this project or if this project has not been designed to comply with the standards or criteria, mark "X" before the appropriate standard or criteria and provide an explanation, including any applicable rule references, in (5)B. below.

ATTACHMENT 2b:
LIFT STATION
(MOPS ENGINEERING REPORT)

ENGINEERING REPORT



PREPARED FOR: MacKenzie Engineering, Inc.
1172 SW 30th Street, Suite 500
Palm City, FL 34990

PROJECT: Advantage Self Storage- Jensen Beach

LOCATION: Jensen Beach (Martin County), FL

DATE: August 30, 2018

AES NO: 1155

Report prepared by:
Bonnie S. McLeod, P.E., Lic # 70797 V.P. of Engineering
Atlantic Environmental Systems, Inc., Certificate # 26398
2244 4th Avenue North, Lake Worth, Florida 33461
Ph: 561-547-8080 Fax: 561-547-3999



ENGINEERING REPORT

TABLE OF CONTENTS

PAGE:	DESCRIPTION:
1155-1	Engineering Report Cover Sheet
1155-2	Table of Contents
1155-3	Project Description and Application
1155-4 - 4.4	Design Compliance
1155-5	Calculation Report Page 1 Primary Design Pressure
1155-6	Calculation Report Page 2 Primary Design Pressure
1155-7	Calculation Report Page 1 Secondary Design Pressure
1155-8	Calculation Report Page 2 Secondary Design Pressure
1155-9	Equivalent Pipe Lengths
1155-10	Buoyancy Calculation Test
1155-11	MOPS Pump Station Schedule
No Page #	Pump Data Sheets / Design Notes / Misc. Attachments



ENGINEERING REPORT

MANUFACTURED ODORLESS PUMP STATION

Project: Advantage Self Storage- Jensen Beach

Location: Jensen Beach (Martin County), FL

Prepared For: MacKenzie Engineering, Inc.
1172 SW 30th Street, Suite 500
Palm City, FL 34990

Date: August 30, 2018

AES Project #: 1155

Notes: Advantage Storage is a proposed self-storage facility with one restroom.

Table One, titled; "For System Design, Estimated Sewage Flows", from sub-chapter 64E6.008, of chapter 64E-6 Titled; "Standards for Onsite Sewage Treatment and Disposal Systems", of the Florida Administrative Code, was used in determining the wastewater flows.

The pump station's total dynamic pump head was based on approximately 75 equivalent feet of 1.5-inch diameter force main. The station discharges into a municipally owned force main with 30 PSI maximum and 10 PSI minimum operating pressure.

SERVICE		GALLONS PER DAY
One Restroom @ 250 GPD/Restroom	=	<u>250</u>
Total Gallons Per Day	=	250

The following pages show the design and sizing parameters considered in the sizing of the wastewater pump stations. In the sizing and design criteria, all minimum required standards for the Florida Department of Environmental Protection Standards were incorporated into the pump station design, including all the requirements outlined in the DEP application form 62-604.300(8)(a).



DESIGN COMPLIANCE CERTIFICATION BASED ON THE FOLLOWING STANDARDS:

**"RECOMMENDED STANDARDS FOR WASTEWATER FACILITIES"
(RSWF), (1997 EDITION)
AND
FLORIDA ADMINISTRATIVE CODE (F.A.C.) RULE 62-604.400
(REVISED 2003)**

Listed below are all the certifications for compliance with the pump station design as it relates to this project.

Certification items 34 through 77 inclusive plus items 78 and 82.

DEP FORM NO: 62-604.300(8)(a) Item Number	F.A.C. (or) RSWF Rule Number	
34	[62-604.400(2)(e) F.A.C.]	The MOPS Engineering Report will show the station's rim is above the 25-year flood level and the control center is above the 100-year flood level. This is in compliance with this rule. This will allow personnel to have access at the 25-year flood level and will not damage any part of the station at the 100-year flood level.
35	[RSWF 41.2]	The MOPS Pump Station is set up to allow easy access to all parts of the system and their components.
36	[RSWF 41.3]	The MOPS Station is designed with round wet well fillet to discourage the accumulation of grit and any piping problems that may be associated with grit build-up.
37	*N/A	This item is not applicable for this pump station design.
38	[RSWF 42.22]	The MOPS Pump Station is designed with submersible pumps with easy guide-out slide rails for the pumps and with "din" rail mounted electrical components for easy maintenance.
39	*N/A	This item is not applicable for this pump station design.
40	[RSWF 42.25]	The MOPS Pump Station utilizes wet well material that is rated for exposure to hydrogen sulfide, corrosive gases, greases, oils, and other products found in wastewater.



- | | | |
|----|------------------------------|---|
| 41 | [RSWF 42.31]
[RSWF 42.36] | The MOPS Pump Station is designed with two pumps. Each pump is sized to pump the full capacity of the application including the application's peak factors. |
| 42 | *N/A | This item is not applicable for this pump station design. |
| 43 | [RSWF 42.33] | The MOPS Pump Station only utilizes pumps that can pass a minimum of 3" solid, or grinder/shredder pumps that will grind the wastewater solids into small fractional particles before transmission into the force main. |
| 44 | [RSWF 42.34] | The MOPS Pump Station, utilizing submersible pumps, always operates under a positive suction head. |
| 45 | [62-604.400(2)(b)
F.A.C.] | The MOPS Pump Station design incorporates a lightning arrestor and surge capacitor as standard equipment. |
| 46 | [RSWF 42.35] | The MOPS Pump Station, in compliance to the NEC code for hazardous location, utilizes pumps rated for either F.M. (Factory Mutual), or Explosion-Proof rated for Class 1, Group "D", Division 1 locations. MOPS Control Centers and control circuits are rated and listed to UL 508A and UL 698A for intrinsically safe use. All the pump station equipment located in the wet well is rated for corrosive type duty service. All MOPS power cables are watertight and sealed in gas tight seal-offs with strain relief. Each MOPS Pump Station is supplied with a fused electrical disconnect switch. All MOPS Pump Station electrical equipment is housed in a N.E.M.A. 4, weatherproof enclosure. Each MOPS Control Center is equipped with a 110-volt power receptacle and ground fault protection. |
| 47 | *N/A | This item is not applicable for this pump station design. |
| 48 | [RSWF 42.38] | The MOPS Engineering Report specifies the station's operating design at peak hourly flows and with a minimum of 2 feet per second flow velocities through the force main. |
| 49 | [RSWF 42.4] | Each MOPS Pump Station Control Center is designed with an alternator to automatically alternate the pumps. |
| 50 | [RSWF 42.5] | MOPS Pump Stations comply with the pertinent requirements for valve type and placement for this rule. |
| 51 | [RSWF 42.62] | MOPS Pump Stations have 30 minutes or less filling volume time, and the pump cycle starts are designed at or below the manufacturer's published maximum. |
| 52 | [RSWF 42.63] | MOPS Pump Stations are designed with a fillet around the bottom of the wet well without interfering with the station's inlet. |



53	[RSWF 42.64]	MOPS Pump Stations are designed to be odorless and provide for air displacement to atmosphere via the scrubber.
54	*N/A	This item is not applicable for this pump station design.
55	*N/A	This item is not applicable for this pump station design.
56	*N/A	This item is not applicable for this pump station design.
57	*N/A	This item is not applicable for this pump station design.
58	*N/A	This item is not applicable for this pump station design.
59	[62-604.400(2)(c) F.A.C.]	MOPS Pump Stations are designed without outside lighting, and they do not create any noises since they use submersible pumps. Included with the station is an exclusive odor scrubber to prevent elimination of odors from the station.
60	[62-604.400(2)(d) F.A.C.]	All MOPS Pump Stations are supplied with keyed-alike padlocks for securing the pump station wet well, valve box, control center and any fencing that is supplied with the project (by others). Each MOPS station is equipped with a maintenance sign to be placed in a visible location for notification of alarms.
61	[RSWF 42.8]	All MOPS Pump Stations are equipped with elapsed time meters.
62	[62-555.30(4) F.A.C. and RSWF 42.9]	The MOPS Pump Stations do not have any connections with potable water supplies. Each MOPS Pump Station is supplied with a U.S.C. and A.S.S.E. approved backflow prevention
63	*N/A	This item is not applicable for this pump station design.
64	*N/A	This item is not applicable for this pump station design.
65	[RSWF 44.1]	MOPS Pump Stations incorporate pumps that are totally submersible, operate within the pump's cycle time, and are in compliance with the NEC code for their use. Each MOPS Pump Station is equipped with mechanical seal fail sensors for each pump to detect shaft mechanical seal failure and potential failure.
66	[RSWF 44.2]	MOPS Pump Stations use submersible pumps that auto-connect to the wet well discharge elbows and slide in and out of the wet well without any dewatering. Pumps are removed without removing any piping.
67	[RSWF 44.31]	MOPS Pump Stations' controls and circuits have strain relief, and are sealed and protected through water and gas tight seals. All equipment can be disconnected from outside the wet well.



68	[RSWF 44.32]	MOPS Pump Stations' controls are enclosed in a weatherproof NEMA 4 enclosure. All conduits associated with application are supplied and installed with appropriate seals and according to the NEC code for Class 1, Group "D", Division 1 Hazardous locations.
69	[RSWF 44.33]	All MOPS Pump Stations are equipped with flexible cords in compliance to the requirement of the NEC. Ground-fault interruption is included. All power terminal fittings and cables are protected and corrosion-resistant.
70	[RSWF 44.4]	MOPS Pump Stations are supplied with valve box assemblies, separate from the wet well. Valve box assemblies either have a piped drain back to the wet well, or have a rock bed bottom for draining water from the box.
71	[RSWF 45]	MOPS Pump Stations are designed with alarms for power failure, pump failure, unauthorized entry, and other pump station malfunctions. The MOPS Pump Station conforms to this rule via two methods: 1) Some stations include a 24 hour monitored telemetered system with instant notification of failure, or, 2) Stations incorporate an audio-visual alarm system with a self contained power back-up source for alarms during power failure.
73	[62-604.40(2)(a) F.A.C. and RSWF 46.431]	MOPS Pump Stations incorporate a riser from the force main for quick connection to portable pumping equipment. The MOPS control center is designed to be compatible with temporary power generation equipment.
74	[RSWF 46.411] [RSWF 46.417] [RSWF 46.432]	MOPS control centers have power protection to protect temporary power equipment once regular power is restored.
75	*N/A	This item is not applicable for this pump station design.
76	*N/A	This item is not applicable for this pump station design.
77	*N/A	This item is not applicable for this pump station design.
78	[RSWF 48.1]	MOPS Pump Stations are designed for a minimum of 2 feet per second force main velocities. All stations have a minimum of 4" force mains (unless a grinder/shredder is specified).
82	[RSWF 48.61]	MOPS Pump Station Engineering Reports utilize 120 for the "C" factor, for pipe friction losses, along with utilizing PVC piping.



DESIGN DEVIATION WITH VARIANCE REQUEST

This engineering report and the pump station design contained herein, based on compliance to all the requirements as outlined in Chapter 62-604 of the Florida Administrative Code with the request for a variance of design as outlined in Rule 62-604.110 Titled: Applicability.

It is the desire of this design to be approved with the following deviations of the standard requirements of the rule, outlined below is the deviation and reason for this deviation:

*See attachment to DEP application for explanation.

Deviation Request:

No. 51: Change (t-avg) maximum pump cycle, from less than 30 minutes to less than 160 minutes:
Rule 62-604.300 Titled "General Technical Guidance" ask for designing cycle in 4.A, Titled "Manual of Practice No: 9". This manual recommends a pump cycle period every 30 minutes or less. This recommendation is based on the assumption that the wastewater entering a pump station starts to become septic and creates H₂S gases and odors in about 20-30 minutes in our Florida climate. This septic wastewater would create an odor problem outside the pump station, through the vent, should the pump cycle time not be met in less than 30 minutes. This pump station will incorporate the M.O.P.S. (Manufactured Odorless Pump Station) designed pump station. This design addresses odor problems from pump stations and is equipped with an odor and gas scrubber; the M.O.P.S. scrubber completely eliminates all mercaptan and gas odor from the pump station, and with the M.O.P.S. scrubber the station would not experience any odor or gas odor problems with a 160 minute cycle time. The scrubber is designed and sized to provide complete odor control for a period of 2 to 3 years, at which time it can be replaced with a replacement odor control scrubber.

PRIMARY CALCULATION SHEET FOR M.O.P.S. WASTEWATER PUMP STATION

Advantage Self Storage- Jensen Beach

LEGEND

WHERE:

Q_{inavg}	= Sewage Flow Rate (influent)
d_w	= Inside diameter of wetwell
D_w	= Outside diameter of wetwell
D_b	= Outside diameter of base
H_b	= Height of base
H_w	= Depth of wetwell
h_{on}	= Lead pump on
h_{off}	= Both pumps off
V_{stor}	= Volume of storage
V_w	= Volume of wetwell
V_b	= Volume of base
V_{wall}	= Volume of wall
V_{earth}	= Volume of earth
Q_{inpeak}	= Peak flow to well
Q_{pump}	= Pump flow (minimum)
	Wetwell basin top elevation
	Wetwell basin bottom elevation
Q_{pump}	= Design point used
	Wear factor
T_{avg}	= Max. pump cycle
T_{min}	= Min. pump cycle
T_{run}	= Min. pump run time
V_1	= Velocity FM 1
V_2	= Velocity FM 2
V_3	= Velocity FM 3

DESIGN:

0.3 GPM
4.00 FT
4.10 FT
7.00 FT
0.10 FT
14.00 FT
3.75 FT
3.25 FT
47.0 GAL
184.7 CU FT
3.9 CU FT
8.9 CU FT
353.8 CU FT
1.3 GPM
13.0 GPM
15.0 FT.
1.0 FT.
13.0 GPM
0.0%
160.3 MIN
14.5 MIN
3.7 MIN
2.4 FPS
0.0 FPS
0.0 FPS

WHERE:

F_{up}	= Weight of up force	11,766 LBS
F_{down}	= Weight of down force	13,302 LBS
AFC_{down}	= Weight of anti-float collar	0 LBS
AFC_{vol}	= Volume of concrete (min)	0 CU YDS
H_{elev}	= Elevation differential	10.8 FT
H_{stat}	= Head in force main	69.3 FT
H_{fric1}	= Friction head FM 1	1.8 FT
H_{fric2}	= Friction head FM 2	0.0 FT
H_{fric3}	= Friction head FM 3	0.0 FT
h_{hi}	= Highest discharge elev.	14.0 FT
L_{pipe1}	= Equiv Length of pipe FM 1	75.0 FT EQUIV
L_{pipe2}	= Equiv Length of pipe FM 2	0.0 FT EQUIV
L_{pipe3}	= Equiv Length of pipe FM 3	0.0 FT EQUIV
d_1	= Foot diameter of pipe FM 1	0.13 FT
d_2	= Foot diameter of pipe FM 2	0.00 FT
d_3	= Foot diameter of pipe FM 3	0.00 FT
C_1	= Pipe roughness factor FM 1	120.0
C_2	= Pipe roughness factor FM 2	0.0
C_3	= Pipe roughness factor FM 3	0.0
P_{fm}	= Pressure in force main	30 PSI
H_v	= Velocity head	0.0 FT (in L_{pipe1})
H_{ps}	= Pump station head loss	0.0 FT (in L_{pipe1})
H_{vv}	= Valve vault head Loss	0.0 FT (in L_{pipe1})
H_{tdh}	= Total dynamic head (calc)	81.9 FT
H_{tdh}	= Design point used	82.0 FT

DESIGN:

FORMULAS

Volumes:

V_{stor}	= $5.872 (d_w)^2 (h_{on} - h_{off})$	47.0 GAL
V_w	= $.785 (D_w)^2 (H_w)$	184.7 CU FT
V_b	= $.785 (D_b)^2 H_b$	3.9 CU FT
V_{wall}	= $.785 \{ (D_w)^2 - (d_w)^2 \} (H_w)$	8.9 CU FT
V_{earth}	= $.785 \{ (D_b)^2 - (D_w)^2 \} (H_w)$	353.8 CU FT

Flotation:

F_{up}	= $(V_b + V_w) \times 62.4$	11766 LBS
AFC_{down}	= $(F_{up} \times 1.1) - (V_{earth} \times 37.6)$	0 LBS
F_{down}	= $(V_{earth} \times 37.6) + AFC_{down}$	13302 LBS
	$F_{down}/F_{up} > 1.1$ (minimum)	Pass Pass/Fail
AFC_{vol}	= $((AFC_{down}/87.6) / 27)$	0.0 Cu. Yds.

Flows:

Q_{inpeak}	Influent x 425% Peak Factor	1.28 GPM
--------------	-----------------------------	----------

Run Times:

T_{avg}	= $\{V_{stor}/(Q_{pump} - Q_{inavg})\} + (V_{stor}/Q_{inavg})$ (<30)	160.3 MIN
T_{min}	= $4(V_{stor}/Q_{pump})$ (>05)	14.5 MIN
T_{run}	= $V_{stor}/(Q_{pump} - Q_{inavg})$ (>02)	3.7 MIN

Velocity:

V_1	= $(2 \times Q_{pump}) / (705 \times d_1^2)$ (>2)	2.36 FPS
V_2	= $(2 \times Q_{pump}) / (705 \times d_2^2)$ (>2)	0.00 FPS
V_3	= $(2 \times Q_{pump}) / (705 \times d_3^2)$ (>2)	0.00 FPS

Head:

H_{elev}	= $h_{hi} - h_{off}$	10.8 FT
H_{stat}	= $P_{fm} \times 2.31$	69.3 FT
H_{fric1}	= $(3.022 \times V_1^{1.85} \times L_{pipe1}) / (d_1^{1.85} \times C^{1.852})$	1.8 FT
H_{fric2}	= $(3.022 \times V_2^{1.85} \times L_{pipe2}) / (d_2^{1.85} \times C^{1.852})$	0.0 FT
H_{fric3}	= $(3.022 \times V_3^{1.85} \times L_{pipe3}) / (d_3^{1.85} \times C^{1.852})$	0.0 FT
H_{tdh}	= $H_{elev} + H_{stat} + H_{fric1} + H_{fric2}$ + $H_{fric3} + H_v^* + H_{ps}^* + H_{vv}^* + wf$	81.9 FT

*These are included in L_{pipe1}



PRIMARY CALCULATION SHEET FOR M.O.P.S. WASTEWATER PUMP STATION

Advantage Self Storage- Jensen Beach

PRIMARY DESIGN PT. = 13 GPM @ 82' TDH (Grinder)

- A) Influent Flow Rate = 250 GPD
 $250 / (60 \times 12) = 0.3$ (12 HOUR DAY)
 0.3 GPM = Avg. Influent rate X 425% Peak Factor = 1.28 GPM
 1.28 GPM = Peak influent RATE
 Pump Design Point Used 13 GPM
- B1) Force Main Size #1 = 2"
 2 FPS = Approx. 12 GPM
 H_{fric1} = see step C
 $V1 @ 13 \text{ GPM} = (2 \times 13) / (705 \times 0.0156) = 2.36 \text{ FPS}$
- B2) Force Main Size #2 = 0"
 2 FPS = Approx. 0 GPM
 H_{fric2} = see step C
 $V2 @ 13 \text{ GPM} = (2 \times 13) / (705 \times 0) = 0 \text{ FPS}$
- B3) Force Main Size #3 = 0"
 2 FPS = Approx. 0 GPM
 H_{fric3} = see step C
 $V3 @ 13 \text{ GPM} = (2 \times 13) / (705 \times 0) = 0 \text{ FPS}$
- C) Total Head = $H_{\text{elev}} + H_{\text{stat}} + H_{\text{fric1}} + H_{\text{fric2}} + H_{\text{fric3}} + H_v + H_{\text{ps}} + H_{\text{vv}} + w_f$
 $H_{\text{elev}} = 10.75'$
 $H_{\text{stat}} = 69.3'$
 $H_{\text{velocity}} = 0'$ (velocity head)
 $H_{\text{station}} = 69.3'$ (misc. station + valve losses)
 $H_{\text{fric1}} = (3.022 \times 2.36^{1.85} \times 75) / (0.125^{1.185} \times 120^{1.852})$
 $= 1.8' \text{ LOSS}$
 $H_{\text{fric2}} = (3.022 \times 0^{1.85} \times 0) / (0^{1.185} \times 0^{1.852})$
 $= 0' \text{ LOSS}$
 $H_{\text{fric3}} = (3.022 \times 0^{1.85} \times 0) / (0^{1.185} \times 0^{1.852})$
 $= 0' \text{ LOSS}$
 F.M.1 length with misc. fittings = 75' equiv. length
 F.M.2 length with misc. fittings = 0' equiv. length
 F.M.3 length with misc. fittings = 0' equiv. length
 Actual TDH = 81.85' + 0% wear factor = 81.85' TDH
 Use 82' TDH
- D) $T_{\text{run}} = V_{\text{stor}} / (Q_{\text{pump}} - Q_{\text{inavg}})$
 $T_{\text{run}} = 2$ minutes minimum
 $2 < V_{\text{stor}} / (13 - 0.3)$
 $V_{\text{stor}} > 25.4 \text{ gal.}$
 $V_{\text{stor Min.}} = (Q_{\text{pump}} - Q_{\text{inavg}}) \times 2 = (13 - 0.3) \times 2 = 25.4 \text{ gal}$
 $T_{\text{avg}} = (V_{\text{stor}} / Q_{\text{pump}} - Q_{\text{inavg}}) + V_{\text{stor}} / Q_{\text{inavg}}$
 $T_{\text{avg}} < 30 \text{ minutes}$
 $\text{Max } V_{\text{stor}} = ((Q_{\text{pump}} - Q_{\text{inavg}}) \times 30 \times Q_{\text{inavg}}) / Q_{\text{pump}}$
 $\text{Max } V_{\text{stor}} = ((13 - 0.3) \times 30 \times 0.3) / 13$
 $\text{Max } V_{\text{stor}} = 8.8$
 $25.4 < V_{\text{stor}} < 8.8$
 Choose 47 gal $V_{\text{stor}} = 0.5'$ depth in 4' dia. wetwell
- E) $T_{\text{avg}} = (47 / (13 - 0.3) + 47 / 0.3)$
 $= 3.70 + 156.59$
 $= 160.29 \text{ minutes}$
 $T_{\text{min}} = 4 \times (47 / 13) = 14.45 > 5 \text{ minutes}$
 $T_{\text{run}} = 47 / (13 - 0.3)$
 $= 3.7 > 2 \text{ minutes}$
- F) $V_{\text{stor}} = 5.872 \times 16 \times 0.5$
 $= 47 \text{ gal}$
 $V_w = .785 \times 17 \times 14$
 $= 184.7 \text{ cu.ft.}$
 $V_b = .785 \times 49 \times 0.1$
 $= 3.85 \text{ cu.ft.}$
 $V_{\text{wall}} = .785 \times (16.81 - 16) \times 14$
 $= 8.9 \text{ cu.ft.}$
 $V_{\text{earth}} = .785 \times (49 - 16.81) \times 14$
 $= 353.8 \text{ cu.ft.}$
 $= 13,302 \text{ lbs weight}$
- G) $F_{\text{up}} = (V_b + V_w) \times 62.4$
 $= (3.85 + 184.7) \times 62.4$
 $= 11,766 \text{ lbs. MAX.}$
 Anti-float collar:
 $= ((F_{\text{up}} \times 1.1) - (V_{\text{earth}} \times 37.6)) / 87.6$
 $= (12942 - 13302) / 87.6$
 $= -4.1051 \text{ cu. Ft.}$
 $= 0 \text{ cu.ft. Concrete (use 0 yds)}$
 $F_{\text{down}} = (V_{\text{earth}} \times 37.6) + \text{antifloat collar}$
 $= 13,302 + 0$
 $= 13,302 \text{ lbs. Min.}$
 Note: $F_{\text{up}} \times 1.1$ is less than or equal to F_{down}
 $= 12942 \leq 13302$
- NOTES: A) Net buoyancy of concrete
 $= 150 \text{ LBS/CU.FT.} - 62.4 \text{ LBS/CU. FT.}$
 $(\text{WATER}) = 87.6 \text{ LBS/CU. FT.}$
 B) Weight of interior equipment not considered
 F_{down} ignores the benefit of the weight of:
 pumps, guide systems, water, basin, etc.

SECONDARY CALCULATION SHEET FOR M.O.P.S. WASTEWATER PUMP STATION

Advantage Self Storage- Jensen Beach

LEGEND

WHERE:

Q_{inavg}	= Sewage Flow Rate (influent)
d_w	= Inside diameter of wetwell
D_w	= Outside diameter of wetwell
D_b	= Outside diameter of base
H_b	= Height of base
H_w	= Depth of wetwell
h_{on}	= Lead pump on
h_{off}	= Both pumps off
V_{stor}	= Volume of storage
V_w	= Volume of wetwell
V_b	= Volume of base
V_{wall}	= Volume of wall
V_{earth}	= Volume of earth
Q_{inpeak}	= Peak flow to well
Q_{pump}	= Pump flow (minimum)
	Wetwell basin top elevation
	Wetwell basin bottom elevation
Q_{pump}	= Design point used
	Wear factor
T_{avg}	= Max. pump cycle
T_{min}	= Min. pump cycle
T_{run}	= Min. pump run time
V_1	= Velocity FM 1
V_2	= Velocity FM 2
V_3	= Velocity FM 3

DESIGN:

0.3 GPM
4.00 FT
4.10 FT
7.00 FT
0.10 FT
14.00 FT
3.75 FT
3.25 FT
47.0 GAL
184.7 CU FT
3.9 CU FT
8.9 CU FT
353.8 CU FT
1.3 GPM
23.0 GPM
15.0 FT.
1.0 FT.
23.0 GPM
0.0%
158.7 MIN
8.2 MIN
2.1 MIN
4.2 FPS
0.0 FPS
0.0 FPS

WHERE:

F_{up}	= Weight of up force	11,766 LBS
F_{down}	= Weight of down force	13,302 LBS
AFC_{down}	= Weight of anti-float collar	0 LBS
AFC_{vol}	= Volume of concrete (min)	0 CU YDS
H_{elev}	= Elevation differential	10.8 FT
H_{stat}	= Head in force main	23.1 FT
H_{fric1}	= Friction head FM 1	5.1 FT
H_{fric2}	= Friction head FM 2	0.0 FT
H_{fric3}	= Friction head FM 3	0.0 FT
h_{hi}	= Highest discharge elev.	14.0 FT
L_{pipe1}	= Equiv Length of pipe FM 1	75.0 FT EQUIV
L_{pipe2}	= Equiv Length of pipe FM 2	0.0 FT EQUIV
L_{pipe3}	= Equiv Length of pipe FM 3	0.0 FT EQUIV
d_1	= Foot diameter of pipe FM 1	0.13 FT
d_2	= Foot diameter of pipe FM 2	0.00 FT
d_3	= Foot diameter of pipe FM 3	0.00 FT
C_1	= Pipe roughness factor FM 1	120.0
C_2	= Pipe roughness factor FM 2	0.0
C_3	= Pipe roughness factor FM 3	0.0
P_{fm}	= Pressure in force main	10 PSI
H_v	= Velocity head	0.0 FT (in L_{pipe1})
H_{ps}	= Pump station head loss	0.0 FT (in L_{pipe1})
H_{vv}	= Valve vault head Loss	0.0 FT (in L_{pipe1})
H_{tdh}	= Total dynamic head (calc)	39.0 FT
H_{tdh}	= Design point used	39.0 FT

DESIGN:

FORMULAS

Volumes:

V_{stor}	= $5.872 (d_w)^2 (h_{on} - h_{off})$	47.0 GAL
V_w	= $.785 (D_w)^2 (H_w)$	184.7 CU FT
V_b	= $.785 (D_b)^2 H_b$	3.9 CU FT
V_{wall}	= $.785 \{ (D_w)^2 - (d_w)^2 \} (H_w)$	8.9 CU FT
V_{earth}	= $.785 \{ (D_b)^2 - (D_w)^2 \} (H_w)$	353.8 CU FT

Flotation:

F_{up}	= $(V_b + V_w) \times 62.4$	11766 LBS
AFC_{down}	= $(F_{up} \times 1.1) - (V_{earth} \times 37.6)$	0 LBS
F_{down}	= $(V_{earth} \times 37.6) + AFC_{down}$	13302 LBS
	$F_{down}/F_{up} > 1.1$ (minimum)	Pass Pass/Fail
AFC_{vol}	= $((AFC_{down}/87.6) / 27)$	0.0 Cu. Yds.

Flows:

Q_{in}	Influent x 425% Peak Factor	1.28 GPM
----------	-----------------------------	----------

Run Times:

T_{avg}	= $\{V_{stor}/(Q_{pump} - Q_{inavg})\} + (V_{stor}/Q_{inavg})$ (<30)	158.7 MIN
T_{min}	= $4(V_{stor}/Q_{pump})$ (>05)	8.2 MIN
T_{run}	= $V_{stor}/(Q_{pump} - Q_{inavg})$ (>02)	2.1 MIN

Head:

H_{elev}	= $h_{hi} - h_{off}$	10.8 FT
H_{stat}	= $P_{fm} \times 2.31$	23.1 FT
H_{fric1}	= $(3.022 \times V^{1.85} \times L_{pipe1}) / (d^{1.85} \times C^{1.852})$	5.1 FT
H_{fric2}	= $(3.022 \times V^{1.85} \times L_{pipe2}) / (d^{1.85} \times C^{1.852})$	0.0 FT
H_{fric3}	= $(3.022 \times V^{1.85} \times L_{pipe3}) / (d^{1.85} \times C^{1.852})$	0.0 FT
H_{tdh}	= $H_{elev} + H_{stat} + H_{fric1} + H_{fric2}$ + $H_{fric3} + H_v^* + H_{ps}^* + H_{vv}^* + wf$	39.0 FT

*These are included in L_{pipe1}

Velocity:

V_1	= $(2 \times Q_{pump}) / (705 \times d_1^2)$ (>2)	4.18 FPS
V_2	= $(2 \times Q_{pump}) / (705 \times d_2^2)$ (>2)	0.00 FPS
V_3	= $(2 \times Q_{pump}) / (705 \times d_3^2)$ (>2)	0.00 FPS



SECONDARY CALCULATION SHEET FOR M.O.P.S. WASTEWATER PUMP STATION

Advantage Self Storage- Jensen Beach

SECONDARY DESIGN PT. = 23 GPM @ 39' TDH (Grinder)

A) Influent Flow Rate = 250 GPD
 $250 / (60 \times 12) = 0.3$ (12 HOUR DAY)
 0.3 GPM = Avg. Influent rate X 425% Peak Factor = 1.28 GPM
 1.28 GPM = Peak influent RATE
 Pump Design Point Used 23 GPM

B1) Force Main Size #1 = 2"
 2 FPS = Approx. 12 GPM
 H_{fric1} = see step C
 $V1 @ 23 \text{ GPM} = (2 \times 23) / (705 \times 0.0156) = 4.18 \text{ FPS}$

B2) Force Main Size #2 = 0"
 2 FPS = Approx. 0 GPM
 H_{fric2} = see step C
 $V2 @ 23 \text{ GPM} = (2 \times 23) / (705 \times 0) = 0 \text{ FPS}$

B3) Force Main Size #3 = 0"
 2 FPS = Approx. 0 GPM
 H_{fric3} = see step C
 $V3 @ 23 \text{ GPM} = (2 \times 23) / (705 \times 0) = 0 \text{ FPS}$

C) Total Head = $H_{\text{elev}} + H_{\text{stat}} + H_{\text{fric1}} + H_{\text{fric2}} + H_{\text{fric3}} + H_v + H_{ps} + H_{vv} + wf$
 $H_{\text{elev}} = 10.75'$
 $H_{\text{stat}} = 23.1'$
 $H_{\text{velocity}} = 0'$ (velocity head)
 $H_{\text{station}} = 23.1'$ (misc. station + valve losses)
 $H_{\text{fric1}} = (3.022 \times 4.176^{1.85} \times 75) / (0.125^{1.85} \times 120^{1.852})$
 $= 5.1' \text{ LOSS}$
 $H_{\text{fric2}} = (3.022 \times 0^{1.85} \times 0) / (0^{1.85} \times 0^{1.852})$
 $= 0' \text{ LOSS}$
 $H_{\text{fric3}} = (3.022 \times 0^{1.85} \times 0) / (0^{1.85} \times 0^{1.852})$
 $= 0' \text{ LOSS}$
 F.M.1 length with misc. fittings = 75' equiv. length
 F.M.2 length with misc. fittings = 0' equiv. length
 F.M.3 length with misc. fittings = 0' equiv. length
 Actual TDH = 38.95' + 0% wear factor = 38.95' TDH
 Use 39' TDH

D) $T_{\text{run}} = V_{\text{stor}} / (Q_{\text{pump}} - Q_{\text{inavg}})$
 $T_{\text{run}} = 2$ minutes minimum
 $2 < V_{\text{stor}} / (23 - 0.3)$
 $V_{\text{stor}} > 45.4 \text{ gal.}$
 $V_{\text{stor Min.}} = (Q_{\text{pump}} - Q_{\text{inavg}}) \times 2 = (23 - 0.3) \times 2 = 45.4 \text{ gal}$

$T_{\text{avg}} = (V_{\text{stor}} / Q_{\text{pump}} - Q_{\text{inavg}}) + V_{\text{stor}} / Q_{\text{inavg}}$
 $T_{\text{avg}} < 30$ minutes
 $\text{Max } V_{\text{stor}} = ((Q_{\text{pump}} - Q_{\text{inavg}}) \times 30 \times Q_{\text{inavg}}) / Q_{\text{pump}}$
 $\text{Max } V_{\text{stor}} = ((23 - 0.3) \times 30 \times 0.3) / 23$
 $\text{Max } V_{\text{stor}} = 8.9$

$45.4 < V_{\text{stor}} < 8.9$
 Choose 47 gal $V_{\text{stor}} = 0.5'$ depth in 4' dia. wetwell

E) $T_{\text{avg}} = (47.0 / (23 - 0.3) + 47.0 / 0.3)$
 $= 2.07 + 156.59$
 $= 158.66$ minutes
 $T_{\text{min}} = 4 \times (47.0 / 23) = 8.17 > 5$ minutes
 $T_{\text{run}} = 47.0 / (23 - 0.3)$
 $= 2.1 > 2$ minutes

F) $V_{\text{stor}} = 5.872 \times 16 \times 0.5$
 $= 47.0 \text{ gal}$
 $V_w = .785 \times 17 \times 14$
 $= 184.7 \text{ cu.ft.}$
 $V_b = .785 \times 49 \times 0.1$
 $= 3.85 \text{ cu.ft.}$
 $V_{\text{wall}} = .785 \times (16.81 - 16) \times 14$
 $= 8.9 \text{ cu.ft.}$
 $V_{\text{earth}} = .785 \times (49 - 16.81) \times 14$
 $= 353.8 \text{ cu.ft.}$
 $= 13,302 \text{ lbs weight}$

G) $F_{\text{up}} = (V_b + V_w) \times 62.4$
 $= (3.85 + 184.7) \times 62.4$
 $= 11,766 \text{ lbs. MAX.}$

Anti-float collar:
 $= ((F_{\text{up}} \times 1.1) - (V_{\text{earth}} \times 37.6)) / 87.6$
 $= (12942 - 13302) / 87.6$
 $= -4.1051 \text{ cu. Ft.}$
 $= 0 \text{ cu.ft. Concrete (use 0 yds)}$

$F_{\text{down}} = (V_{\text{earth}} \times 37.6) + \text{antifloat collar}$
 $= 13,302 + 0$
 $= 13,302 \text{ lbs. Min.}$

Note: $F_{\text{up}} \times 1.1$ is less than or equal to F_{down}
 $= 12942 \leq 13302$

NOTES: A) Net buoyancy of concrete
 $= 150 \text{ LBS/CU.FT.} - 62.4 \text{ LBS/CU. FT.}$
 $(\text{WATER}) = 87.6 \text{ LBS/CU. FT.}$
 B) Weight of interior equipment not considered
 F_{down} ignores the benefit of the weight of:
 pumps, guide systems, water, basin, etc.

EQUIVALENT PIPE LENGTHS

Advantage Self Storage - Jensen Beach

MOPS

FITTING	MATERIAL	PIPE SIZE IN INCHES															FORCE MAIN RUN #1			FORCE MAIN RUN #2			FORCE MAIN RUN #3		
		1	1 1/4	1 1/2	2	2 1/2	3	4	6	8	10	12	14	16	QTY	TOTALS	1 1/2								
90° Elbow	PVC	4	4	4	6	7	8	11	16	21	26	32	37	43		0 ft									0 ft
	HDPE ID	0	0	0	0	0	0	0	0	0	0	0	0	0		0 ft									0 ft
45° Elbow	Ductile Iron	0	0	0	0	0	4	5	8	10	12	15	17	19		0 ft									0 ft
	PVC	2	2	3	4	4	4	5	8	11	14	16	18	20		0 ft									0 ft
22 1/2° Elbow	HDPE ID	0	0	0	0	0	0	0	0	0	0	0	0	0		0 ft									0 ft
	Ductile Iron	0	0	0	0	0	3	4	5	7	8	10	12	13		0 ft									0 ft
11 1/4° Elbow	PVC	2	2	3	3	4	4	5	5	6	7	9	11	12		0 ft									0 ft
	HDPE ID	0	0	0	0	0	0	0	0	0	0	0	0	0		0 ft									0 ft
Run Tee	Ductile Iron	0	0	0	0	0	2	3	3	4	4	5	6	7		0 ft									0 ft
	PVC	3	3	3	4	5	6	8	12	14	18	20	25	27		0 ft									0 ft
Branch Tee	HDPE ID	0	0	0	0	0	0	0	0	0	0	0	0	0		0 ft									0 ft
	Ductile Iron	0	0	0	0	0	2	3	4	4	5	6	7	8		0 ft									0 ft
Ball/Gate Valve	PVC	8	8	9	12	15	16	22	33	49	57	67	78	88		0 ft									0 ft
	HDPE ID	0	0	0	0	0	0	0	0	0	0	0	0	0		0 ft									0 ft
Check Valve (Swing or Ball)	Ductile Iron	0	0	0	0	0	8	10	15	20	25	30	35	39		0 ft									0 ft
	PVC or Bronze	2	2	3	3	3	3	3	4	4	4	4	4	4		0 ft									0 ft
RUNNING LENGTH OF FORCE MAIN	Ductile Iron	13	13	15	19	22	27	38	52	74	98	120	0	0	1	15 ft									0 ft
	PVC	13	13	15	19	22	27	38	52	74	98	120	0	0		0 ft									0 ft
MOPS Valve Vault Losses		30	30	34	43	50	57	77	107	151	156	189	213	245		34 ft									
MOPS Wetwell Losses																16 ft									
L _{pipe1} , L _{pipe2} , and L _{pipe3}																	10 ft						0 ft		
Valve Vault Losses = 1 G/BV, 1CV, 1 90° Elbow, 1 Branch Tee, + 3ft Pipe (All PVC)																									
Wet Well Losses = 2 90° Elbows, Depth -2ft Vertical Pipe, + 4ft Horiz. Pipe (All PVC)																									

BUOYANCY CALCULATIONS - FIBERGLASS WET WELL

GOAL: Downward force F_{down} , from weight of earth > Upward force F_{up} , from water buoyancy, by at least 10% (that is $F_{down} > F_{up} * 1.1$).

Assumptions:

1. Water weight = 62.4 lbs./ cu.ft.
2. Fiberglass weight = Not Considered
3. Soil Weight, when submerged in water = 37.6 lbs./ cu ft.
4. Fiberglass weight when submerged in water = 0
5. The hatch cover area is not used in calculating F_{up} or F_{down} .
6. The weight of any equipment or piping is not used in figuring F_{down} .
7. Vearth (the weight of the volume of compacted soil, submerged in water, above the outside diam. of the wet well base) is used in calculating F_{down} .
8. No shear / friction forces or 'cone effects' are considered for F_{down} .
9. F_{down} calculation may include any required concrete anti-flotation collar.
10. Calculations are based on the wet well being fully empty and the water table being at ground elevation level.

All Calculations are based on Current figures in report.

Please check the "Formulas & Constants" sheet for reference.

F_{up} =	11,766 LBS
F_{down} =	13,302 LBS
Safety Factor	1.1
Is $F_{down} > F_{up} * 1.1$	PASS

CONCLUSION:

$V_{requiredConcrete} = 0$ cu.ft. Concrete (use 0 yds)

If $V_{requiredConcrete} = 0$ then no anti-flotation collar is required. If $V_{requiredConcrete}$ is > 0 then a concrete collar in the amount of $V_{requiredConcrete}$ must be used for the anti-flotation collar.

Based on the conclusions above, the fiberglass wet well, as designed, will not experience any flotation problems, even under the worst cases. The Buoyancy safety factor is in excess of 10%.

MOPS PUMP STATION SCHEDULE

MOPS SERIES		ITEM DESCRIPTION	
PRIMARY DESIGN FLOW (G.P.M.)	13	100 YEAR FLOOD ELEVATION	14.00'
PRIMARY DESIGN HEAD (T.D.H.)	82	25 YEAR FLOOD ELEVATION	12.17'
SECONDARY DESIGN FLOW (G.P.M.)	23	A GRADE ELEVATION	15.00'
SECONDARY DESIGN HEAD (T.D.H.)	39	B TOP ELEVATION OF WET WELL	15.00'
RATED PERFORMANCE SPEED	3450	C BOTTOM ELEVATION OF WET WELL	1.00'
RATED MOTOR HORSEPOWER	3	D ALL PUMPS OFF ELEVATION	3.25'
SUBMERSIBLE PUMP TYPE (P-1,P-2)	GRINDER	E LEAD PUMP ON ELEVATION	3.75'
PUMP MODEL NUMBER	MOPS	F LAG PUMP ON ELEVATION	4.25'
SERVICE ENTRANCE VOLTAGE	230	G HIGH ALARM ELEVATION	4.75'
SERVICE ENTRANCE PHASE	3	H INSIDE DIAMETER OF WET-WELL	48"
CONTROL CENTER FULL LOAD AMPS	29	I OUTSIDE DIAMETER OF WET WELL'S ANTI-FLOTATION FLANGE	84"
NEMA 3R PAINTED STEEL DISCONNECT SWITCH, RATED AMPS	30	J MINIMUM CUBIC FEET OF CONCRETE BALLAST COLLAR (CU YDS)	0/(0)
WET-WELL SCOURER SYSTEM	N/A	K INVERT PIPE DIAMETER	4"
REMOTE STATION MONITOR (TELEMETRY)	W/SA	L FORCE MAIN DIAMETER	1.50"
ON-SITE GENERATOR SYSTEM	N/A	M INVERT ELEVATION	5.25'
		N SECONDARY INVERT ELEVATION	N/A

MOPS EQUIPMENT IDENTIFICATION	QTY.	MODEL DESIGNATION
MOPS PUMP STATION	1	B21.5-48168-C-3.0
MOPS VALVE BOX ASSEMBLY (VBA)	1	VBA-21.5
MOPS ODOR/GAS SCRUBBER (OGS)	1	OGS-B
MOPS R.P.Z. ASSEMBLY	1	75
MOPS CONTROL CENTER	1	PSC-222-3.0
MOPS DISCONNECT SWITCH	1	FDS-30-3-2-PS
MOPS CONTROL CENTER MOUNTING ASSEMBLY	1	CCMA-32AL
MOPS WET WELL SCOURER SYSTEM	0	N/A
MOPS REMOTE STATION MONITOR	1	PROVIDED WITH SERVICE AGREEMENT
1ST YEAR SERVICE / MAINTENANCE CONTRACT	1	LEVEL 1 WITH REMOTE MONITOR

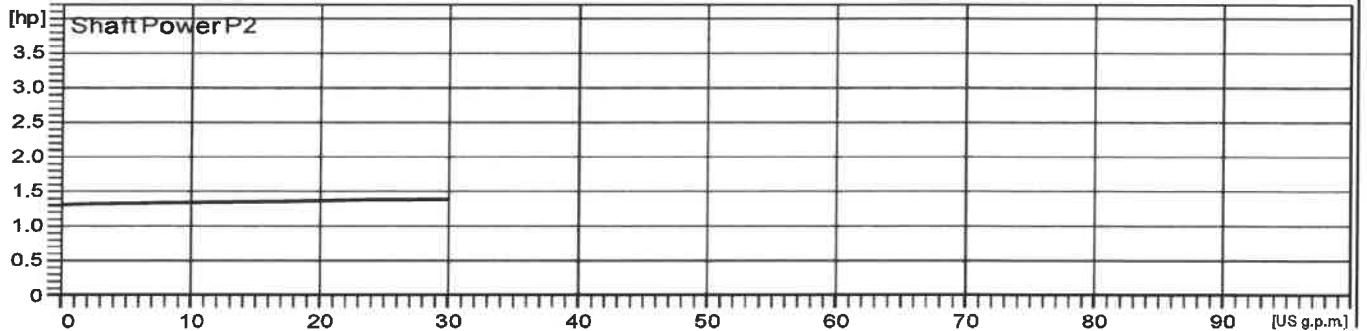
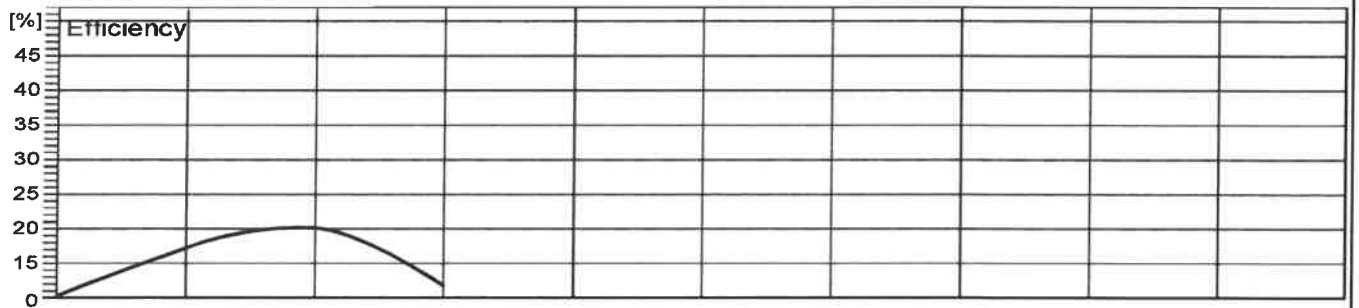
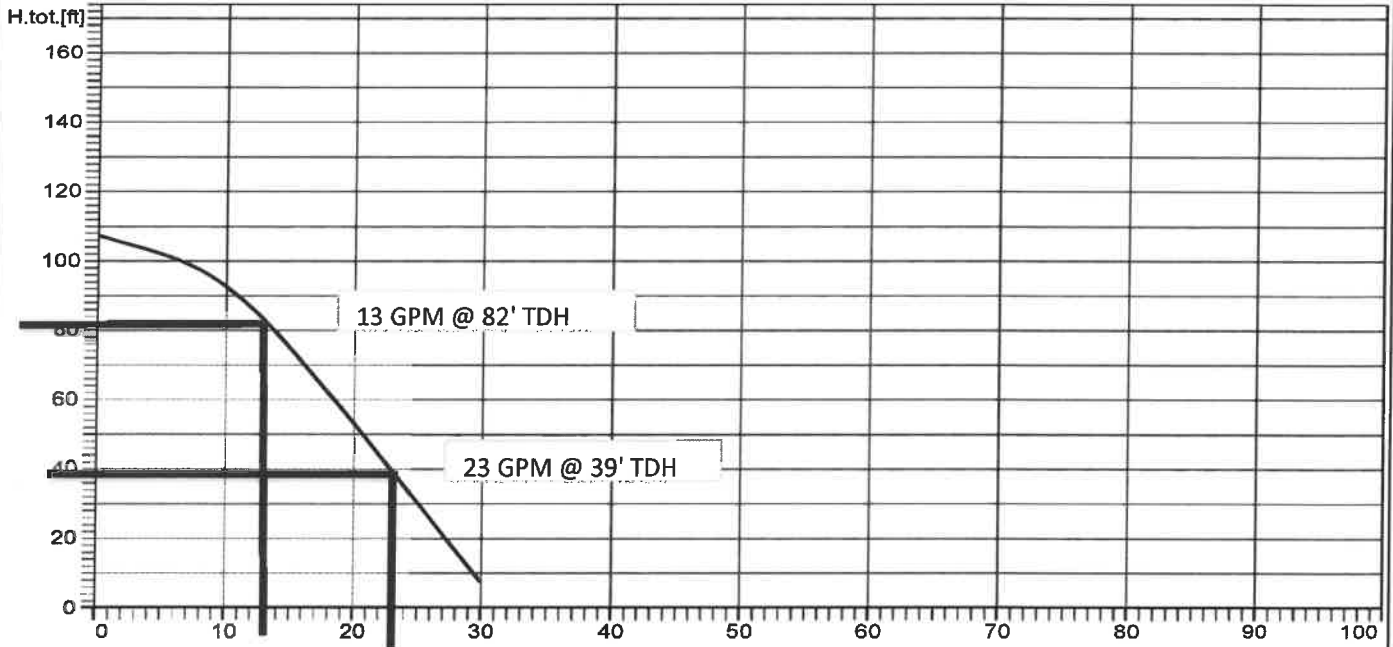
All Design Parameters are job specific (See Drawing)

Performance Curve

GN132XN & GN332/4XN

MOPS

Impeller					
Impeller type: Recessed vortex vane impeller with grinder sys.		Solid Size Ø: Grinder	Max. Ø: 6 ½"	Min. Ø: 4 ½"	Sel. Ø: 6 ½"
Phase: 1 / 3					
Operating data					
Speed: 3450 rpm	Frequency: 60 Hz	13 GPM @ 82' TDH; 23 GPM @ 39' TDH		Pump Station: Duplex / Simplex	Discharge Port: 2" Male NPT
Power data referred to: (STP) Water [100%] ; 68°F; 62.4 lbs/cu.ft.; 31 SSU; 1.0 S.G.			Test Standard: ISO9906	Max. Motor HP: 3.0 HP	



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MOPS Grinder Pump Data Sheet

MOPS

2" Nominal Discharge
0.25" Max. Solid Size Output
2-poles; 3450 RPM



Open Multi-Vane Vortex
Impeller With Grinder System

Model
GN13 1-Phase
GN33 3-Phase

Electrical Data 1-Phase

Model Number	Voltage	Phase	Nominal Rated H.P.	AMPS F.L.	AMPS L.R.	Power Factor	Motor Eff.(%)	Cable Size	Cable O.D. (in)
GN132XO	200	1	3.0	21.6	54.0	0.95	85	10GA	0.75
GN132XO	208	1	3.0	20.8	52.0	0.95	85	10GA	0.75
GN132XO	230	1	3.0	18.8	47.0	0.95	85	10GA	0.75

Electrical Data 3-Phase

Model Number	Voltage	Phase	Nominal Rated H.P.	AMPS F.L.	AMPS L.R.	Power Factor	Motor Eff.(%)	Cable Size	Cable O.D. (in)
GN332XO	200	3	3.0	11.4	68.0	0.85	85	10GA	0.75
GN332XO	208	3	3.0	10.9	65.0	0.85	85	10GA	0.75
GN332XO	230	3	3.0	9.9	59.0	0.85	85	10GA	0.75
GN332XO	460	3	3.0	5.0	30.0	0.85	85	10GA	0.75

Motor Construction

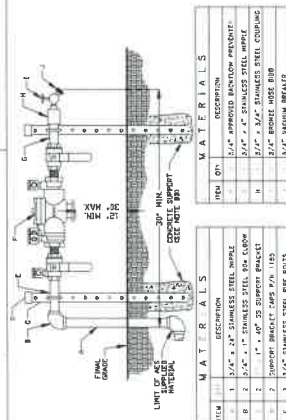
Motor Type	Enclosed Submersible
NEMA Insulation Code	Class F
Service Factor	1.15
NEMA Design Letter	B
Standard Cable Length	30 ft
Available Motor Voltages	1-Phase: 200-230 / 3-Phase: 200-460
Thermal Sensors embedded in Windings	Included in all Pumps. Must be Wired into Control Panel to Validate Warranty.
Explosion Proof Construction	Class I, Div 1, Groups C & D - Explosion Proof Rated
Single Phase Units Require Drives in Control Center.	

Standard Material of Construction

Motor Housing	Cast Iron ASTM A48; Class 40B
Volute	Cast Iron ASTM A48; Class 40B
Impeller	Cast Iron ASTM A48; Class 40B
Grinder Assembly	Hardened Stainless Steel 55 Rockwell C
Shaft	AISI 430F Stainless Steel
Mechanical Seal - Impeller Side	Silicon Carbide vs Silicon Carbide
Mechanical Seal - Motor Side	Silicon Carbide vs Silicon Carbide
Fasteners	AISI 304 Stainless Steel
O-Rings	Nitrile Rubber
Upper Bearing	Deep Groove Ball Bearing
Lower Bearing	Double Row Angular Ball Bearing
Power Cable Sheathing	Nitrile Rubber

ATTACHMENT 2b:
LIFT STATION
(Lift Station Design)

DETAIL B



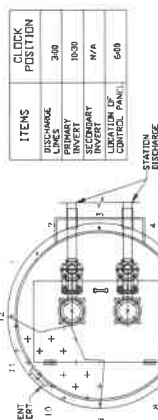
CONDUITS

CONTROL PANEL

6

NOTE: BACKFILL AROUND LIFT STATION FROM INSIDE COORDINATION SHALL BE CONFINED TO GRADE SHALL BE CONFINED TO COMPACTED IN 12-INCH MAX LIFTS TO 98% DENSITY PER ASTM D 1585. CONTRACTOR SHALL I-100. CONTRACTOR SHALL EXERCISE CARE NOT TO DISTURB

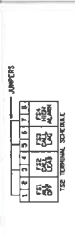
HEMATIC BASED ON THE STATION'S DISCHARGE BEING CONTROLLED AT THE #2 CLOCK POSITION. SEE SCHEDULE 1 FOR LOCATION OF INVERTS AND CONTROL PANEL.



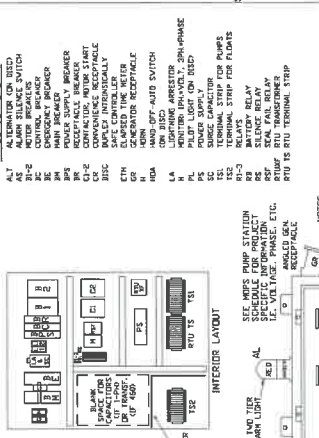
HUB AND INVERT LOCATION	
1	2
3	4
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99	100

MOPS CONTROL CENTER

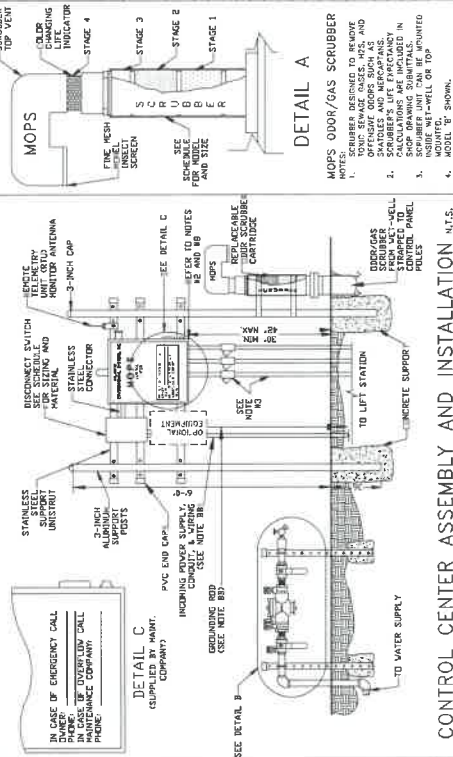
L D.E.P. REQUIREMENTS.
FOR ELECTRICAL SERVICE
REQUIREMENTS.



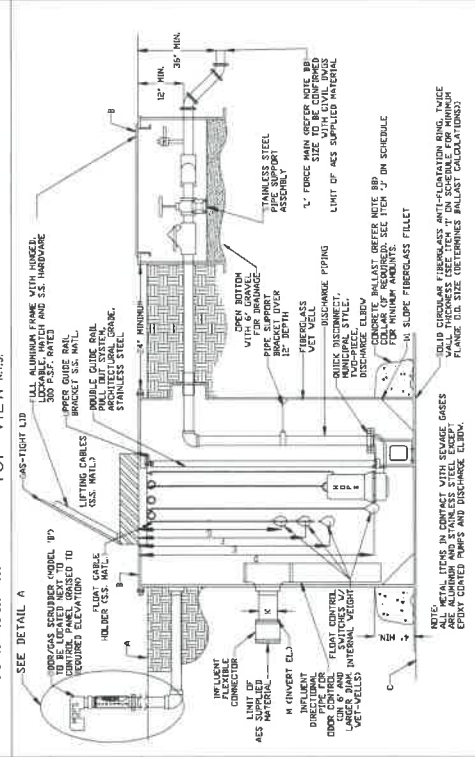
NOTES:
A. 1/4" X 1/4" STAINLESS STEEL ENCLOSURE
B. ALUMINUM INNER (GAS PRVNT) DOOR 30TH
X 24V X B7D
C. PANEL LABELED FOR U.L. 300BA "MOTOR
CONTROL CENTERS" AND VAL. 599A
"HAZARDOUS LOCATION PANELS"
D. INTERNALLY SAFED
E. BATTERY VISION-ALARM SYSTEM,
PER 55W-45



CONTROL CENTER ASSEMBLY AND INSTALLATION



TOP VIEW NTS



SECTION VIEW N.T.S.

NOTE:
ALL METAL ITEMS IN CONTACT WITH SEWAGE GASES
ARE ALUMINUM AND STAINLESS STEEL EXCEPT
EPOXY COATED PUMPS AND DISCHARGE CLIMB.
—SOLID CIRCULAR FIBERGLASS ANTI-FLOTATION RING, TWICE
WALL THICKNESS (SEE ITEM 'I' ON SCHEDULE FOR MINIMUM
FLANGE O.D. SIZE (DETERMINES BALLAST CALCULATIONS))

MOPS ENGINEERING NOTES:

THE HORSEPOWER SHOWN ON THE SCHEDULE IS A MINIMUM HORSEPOWER REQUIREMENT BASED ON THE STATION'S DESIGN CRITERIA AND THE REQUIRE TORQUE. (LOWER RATED HORSEPOWER EQUIPMENT WILL NOT BE ACCEPTABLE.)

A. BY THE MOPS STATION.

B. BY A.E.S. WITH THE MOPS STATION.

C. INVERT ELEVATIONS BASED ON INSIDE BOTTOM OF PIPE.

D. THE MOPS CONTROL ASSEMBLY CONSISTS OF THE FOLLOWING: CONTROL CENTER, DISCONNECT SWITCH, INVERT VALVE, AND INVERT STOP VALVE.

E. INSTALLED BY THE MOPS PUMP STATION MANUFACTURER TO VALIDATE MOPS WARRANTY PROGRAM.

F. THE MOPS WASTEWATER PLANT STATION DESIGN AND EQUIPMENT SHOWN ON THIS DRAWING HAS BEEN REVIEWED, PERMITTED, AND CERTIFIED AS COMPLYING WITH ALL THE STATE OF FLORIDA REGULATORY AGENCIES' REQUIREMENTS FOR WASTEWATER TREATMENT PLANTS.

G. APPLICATION FEES, AND ENGINEERING SERVICES FOR RE-CERTIFICATION AND DESIGN REVIEW.

MOPS ENGINEERING NOTES
SITE CIVIL ENGINEER.

1. PUMPS ARE RATED BY FACTORY MUTUAL FOR CLASS 1, GROUP D ATMOSPHERES AS REQUIRED BY NEC.
2. THE CONTROL CENTER INCORPORATES INTRINSICALLY SAFE RELAYS AND IS LISTED TO UL 598A FOR USE IN HAZARDOUS LOCATIONS.
3. THE CONDUIT PROVIDED, ALONG WITH CONDUIT GAS-SEAL-OFFS, ARE RATED FOR CLASS 1, DIVISION 1 LOCATIONS.
4. WASTEWATER PUMPS AND THE CONTROL CENTER INCORPORATE A MECHANICAL SEAL FAILURE DETECTION AND NOTICE SYSTEM.
5. THE CONTROL CENTER INCLUDES EITHER A REMOTE TELEMETRY UNIT (RTU) OR A SELF-CHARGING, BACK-UP ALARM SYSTEM TO OPERATE ON POWER FAILURE.
6. THE PUMP STATION INCORPORATES AN ADDRESSLESS DESIGN WITH A SCRUBBER SYSTEM TO CONTROL EMISSIONS.
7. THE BOTTOM OF THE TOP ROW ELEVATION OF PUMP STATION MUST BE LOCATED AT A HIGHER ELEVATION THAN THE 25 YEAR FLOOD ELEVATION. THE LISTED 25 YEAR FLOOD ELEVATION PROVIDED BY STATE OF TEXAS ENGINEER.
8. THE BOTTOM ELEVATION OF THE HOPS CONTROL CENTER MUST BE LOCATED AT A HIGHER ELEVATION THAN THE 100 YEAR FLOOD ELEVATION. THE LISTED 100 YEAR FLOOD ELEVATION PROVIDED BY THE STATE OF TEXAS ENGINEER.

PROJECT FIELD SERVICE WORK	CONTINUED
MOPS PUMP STATION COMPLIANCE NOTES	

THIS PUMP STATION DESIGN COMPLIES WITH THE FOLLOWING REQUIRED STANDARDS:
STATE OF FLORIDA ENVIRONMENTAL PROTECTION STANDARDS

- FLORIDA ADMINISTRATIVE CODE (F.A.C.): 62-640.400- COLLECTION AND TRANSMISSION SYSTEMS
NATIONAL ELECTRIC CODE (NEC) CLASS 1, DIVISION 1, GROUP D- HAZARDOUS LOCATIONS
UNDERWRITER'S LABORATORIES (U.L.) 508A-MOTOR CONTROL CENTERS AND U.L. 698A-INSTRINSICALLY
SAFE CONTROL CENTERS

MOPS EQUIPMENT IDENTIFICATION		QTY.	MODEL DESIGNATION
MOPS PUMP STATION		1	BP1.5-48158-C-3.0
MOPS VALVE BOX ASSEMBLY (VBA)		1	VBA-271.5
MOPS ODR/OAS SCRUBBER (OGS)		1	OGS-27.5
MOPS R.P.Z. ASSEMBLY		1	75
MOPS CONTROL CENTER		1	PSC-222-3.0
MOPS DISCONNECT SWITCH		1	FD9-10-3-2-PS
MOPS CONTROL CENTER MOUNTING ASSEMBLY		1	CCMA-32AL
MOPS WET WELL SCOURER SYSTEM		0	N/A
MOPS REMOTE STATION MONITOR		1	PROVIDED WITH SERVICE AGREEMENT
1st YEAR SERVICE/MAINTENANCE CONTRACT		1	LEVEL 1 WITH REMOTE MONITOR
MOPS ON-SITE GENERATOR SYSTEM		0	N/A
MOPS FIELD SERVICE WORK		1	CONTROL INSTALLATION & START-UP

MOPS PUMP STATION SCHEDULE

LAKE WORTH, FL 33461
PH: (561) 547-8080 FAX: (561) 547-3999 © 2000

MANUFACTURED ODORLESS PUMP STATION
ATLANTIC ENVIRONMENTAL SYSTEMS, INC.

MOPS

PRIVATE SERIES

THIS DRAWING AND THE DESIGN CONTAINED HEREIN IS PROPRIETARY AND IS AND SHALL REMAIN THE PROPERTY OF ATLANTIC ENVIRONMENTAL SYSTEMS, INC. THIS DRAWING AND DESIGN SHOULD BE USED ONLY FOR THE PURPOSES FOR WHICH IT IS INTENDED, AND ONLY WITH WRITTEN AUTHORIZATION FROM ATLANTIC ENVIRONMENTAL SYSTEMS, INC. ANY REPRODUCTION OF THIS DRAWING OR DESIGN FOR ANY OTHER ATLANTIC ENVIRONMENTAL SYSTEMS, WITHOUT VIOLATE ATLANTIC ENVIRONMENTAL SYSTEMS' COPYRIGHTS.

REVISION	DATE
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11/16/17	Deliv.
1155	Project: No:

Page No.
 LS-1

ADVANTAGE SELF STORAGE - JENSEN BEACH
MACKENZIE ENGINEERING AND PLANNING, INC.
1172 SW 30TH ST., SUITE 500, PALM CITY, FL

**ATTACHMENT 3:
IRRIGATION
(SUMMARY OF
GROUNDWATER – WELL
FACILITY)**

SECTION IV – SOURCES OF WATER

SUMMARY OF GROUNDWATER (WELL) FACILITIES

Well Name or Number	1 of 1					
Map Designation						
Existing or Proposed	Proposed					
Date of Proposed Construction	N/A					
Date Installed if Existing	N/A					
Diameter (in)	2"					
Total Depth (ft)	130'					
Cased Depth (ft)	105'					
Screened Interval (ft)	105' - 130'					
Pumped or Flowing	Pumped					
Pump Type (see Instructions)	1 1/2 HP Centrifugal					
Pump Intake Depth (ft bls)	Surface					
Pump or Flow Capacity (GPM)	50 GPM					
Working Valve if Artesian (yes, no or not applicable)	N/A					
Status (see Instructions)	Primary					
Purpose (see Instructions)	Irrigation					
Elevation of the Wellhead (ft NGVD - see Instructions)	N/A					
Water Use Accounting Method (see Instructions)	N/A					
Date Last Calibrated (ATTACH calibration report)	N/A					
Planar Coordinates (if known - see instructions)	N/A					
Section / Township / Range	21/37S/41E					

Instruction for Completing Groundwater (Wells) Section

Well Name or Number: The Applicant's designation of the well. How do you refer to it?

Map Designation: This is how the well is labeled on the map submitted with the application. This may be the same as Well Name or Number, but does not necessarily have to be.

Existing or Proposed: If the well is proposed, enter the date of expected operation. If it is an existing well, enter the date it was installed if you know it.

Diameter: Outside diameter of the well casing.

Total Depth: Total length in feet between the land surface and the bottom of the well.

Cased Depth: The length in feet from the land surface to the bottom of the well casing.

Screened Interval: The distance in feet below land surface to the top and bottom of the well screen, if the well is so equipped.

Pumped or Flowing: Does the well produce water as a result of natural artesian flow, or is it pumped?

Pump Type: This is the type of pump that has been installed for the well (typical choices are as follows):

Centrifugal	Diesel turbine	Axial flow	Windmill
Submersible	Jet	Suction	Other (specify)
Electric turbine	Hydraulic	Portable	

Pump Intake Depth: Location of the pump depth in feet below land surface. The pump may be on the surface or down inside the well.

Pump or Flow Capacity: The amount of water the pump can produce in gallons per minute (GPM).

Working Valve: If the well is artesian, does it have a working valve to control the flow?

Status: Primary
 Secondary (i.e. a production well that is rotated)
 Standby (i.e. used for freeze protection or emergency)
 Monitor
 Injection (i.e. A/C, pool heat exchange, etc.; sometimes used only periodically)
 Recharge (i.e. same as above)

Purpose: What will the water be used for (typical choices are as follows):

Dairy	Irrigation	Air Conditioning	Swimming Pool Heating
Monitor	Aquaculture	Freeze Protection	Irrigation/Lake Recharge
Livestock	Bottled Water	Mining/Dewatering	Aquifer Storage and Recovery
Industrial	Other (specify)	Public Water Supply	Aquifer Remediation and Recovery

Elevation of the Wellhead: This is the elevation of the top of the finished well at the ground surface.

Planar coordinates: The Florida State Plane System (Planar Coordinates) should be submitted if you have a land survey which identifies the location of the well in terms of those measurements. If you do not know what these are, it is not necessary to include them.

Section / Township / Range: The section, township and range in which the pump is located.

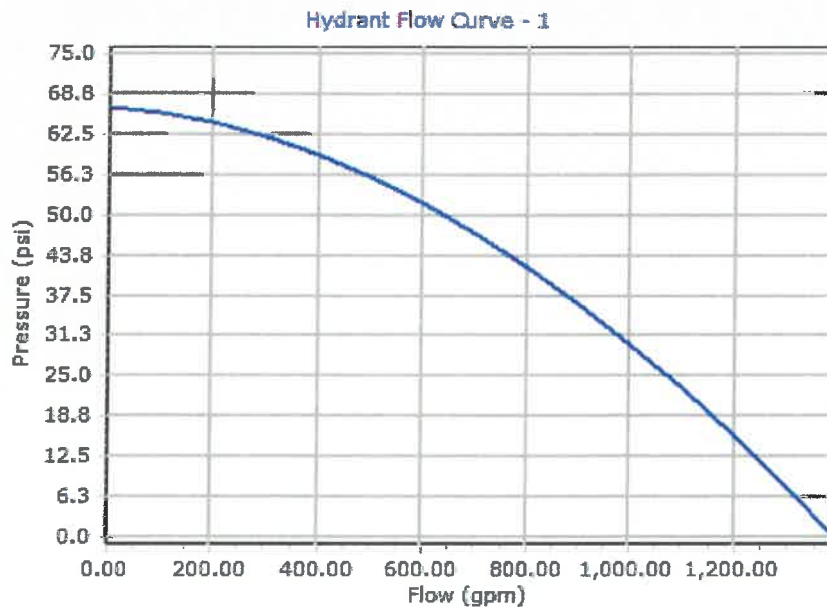
ATTACHMENT 4:
FIRE FLOW
(EXISTING FIRE HYDRANT
FLOW)

Hydrant Flow Curve Detailed Report - Hydrant Flow Curve - 1

Element Details			
Label	Hydrant Flow Curve - 1	Nominal Hydrant Flow	1,000.00 gpm
Hydrant/Junction	FH63	Number of Intervals	10

Time (hours)
0.000

0.000 hours Flow (gpm)	0.000 hours Pressure (psi)
0.00	66.7
100.00	65.9
200.00	64.4
300.00	62.2
400.00	59.3
500.00	55.9
600.00	51.8
700.00	47.2
800.00	41.9
900.00	36.2
1,000.00	29.9
1,100.00	23.0
1,200.00	15.6
1,300.00	7.7
1,391.62	0.0

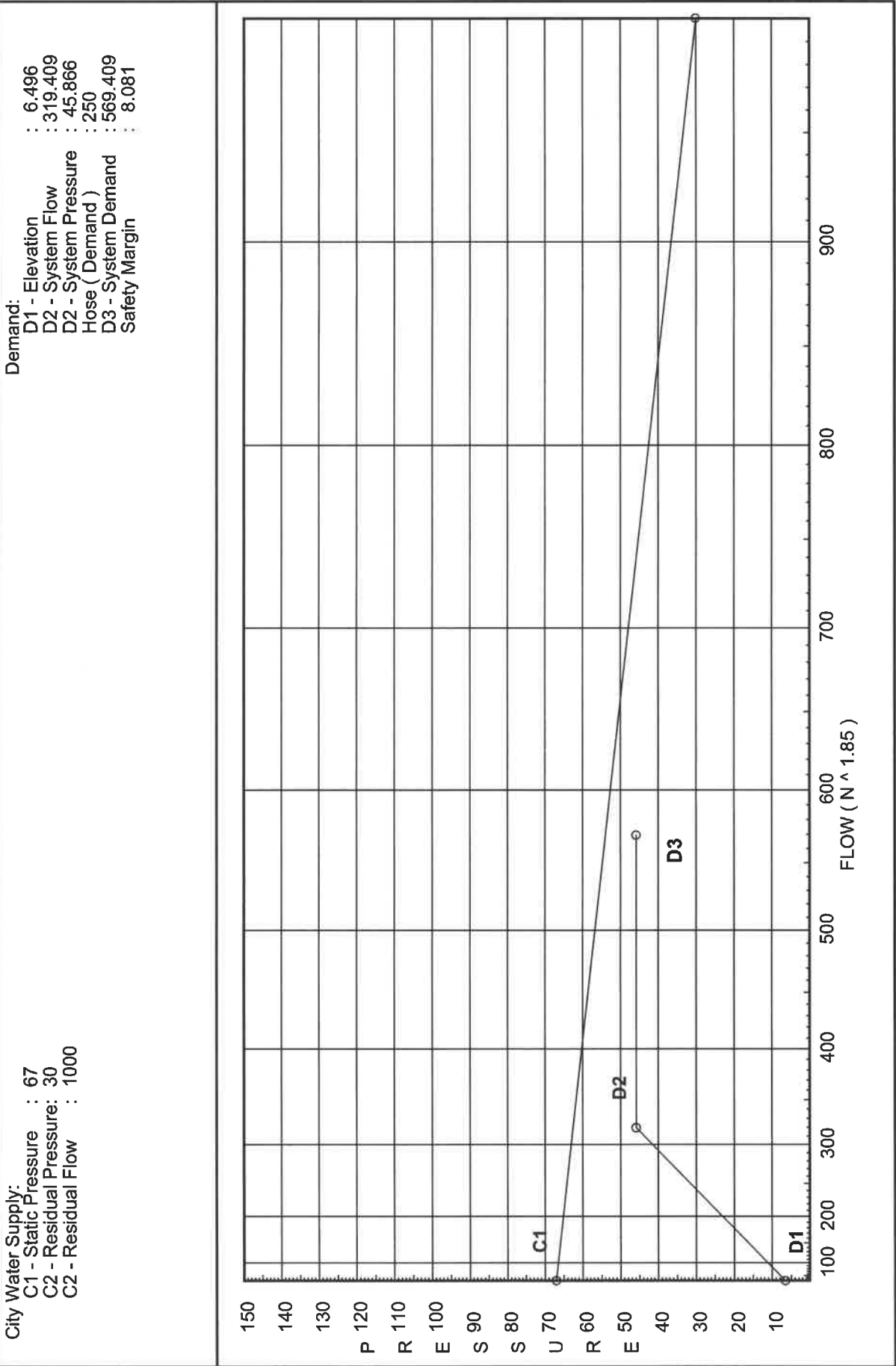


ATTACHMENT 4:
FIRE FLOW
(FIRE FLOW CALCULATION)



RED HAWK FIRE & SECURITY
3150 SW 42ND AVE
PALM CITY, FL. 34990
(772)232-2001

Job Name : Estimate Test
Building :
Location :
System :
Contract :
Data File : ADVANTAGE.WX1



Fittings Used Summary

RED HAWK FIRE & SECURITY
Estimate Test

Fitting Legend Abbrev. Name	1/2	3/4	1	1 1/4	1 1/2	2	2 1/2	3	3 1/2	4	5	6	8	10	12	14	16	18	20	24
C Roll Groove Coupling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
E NFPA 13 90' Standard Elbow	1	2	2	3	4	5	6	7	8	10	12	14	18	22	27	35	40	45	50	61
G NFPA 13 Gate Valve	0	0	0	0	0	1	1	1	1	2	2	3	4	5	6	7	8	10	11	13
T NFPA 13 90' Flow thru Tee	3	4	5	6	8	10	12	15	17	20	25	30	35	50	60	71	81	91	101	121
Zab Ames 2000SE	Fitting generates a Fixed Loss Based on Flow																			

Unit Summary

Diameter Units Inches
Length Units Feet
Flow Units US Gallons per Minute
Pressure Units Pounds per Square Inch

Note: Fitting Legend provides equivalent pipe lengths for fittings types of various diameters. Equivalent lengths shown are standard for actual diameters of Sched 40 pipe and CFactors of 120 except as noted with *. The fittings marked with a * show equivalent lengths values supplied by manufacturers based on specific pipe diameters and CFactors and they require no adjustment. All values for fittings not marked with a * will be adjusted in the calculation for CFactors of other than 120 and diameters other than Sched 40 per NFPA.

Pressure / Flow Summary - STANDARD

RED HAWK FIRE & SECURITY
Estimate Test

Page 3
Date 012501

Node No.	Elevation	K-Fact	Pt Actual	Pn	Flow Actual	Density	Area	Press Req.
1	15.0	5.6	21.56	na	26.0	0.2	130	7.0
2	15.0	5.6	21.62	na	26.04	0.2	130	7.0
3	15.0	5.6	21.85	na	26.18	0.2	130	7.0
5	15.0	5.6	22.22	na	26.39	0.2	130	7.0
6	15.0	5.6	22.28	na	26.43	0.2	130	7.0
7	15.0	5.6	21.6	na	26.03	0.2	130	7.0
8	15.0	5.6	21.67	na	26.07	0.2	130	7.0
9	15.0	5.6	21.9	na	26.2	0.2	130	7.0
11	15.0	5.6	22.26	na	26.42	0.2	130	7.0
12	15.0	5.6	22.33	na	26.46	0.2	130	7.0
15	15.0	5.6	26.03	na	28.57	0.2	130	7.0
16	15.0	5.6	26.1	na	28.61	0.2	130	7.0
4	15.0		22.69	na				
10	15.0		22.74	na				
14	15.0		26.57	na				
17	15.0		27.29	na				
18	15.0		27.35	na				
19	15.0		27.57	na				
20	15.0		31.93	na				
21	15.0		34.37	na				
22	0.0		41.12	na				
23	0.0		45.87	na	250.0			

The maximum velocity is 18.94 and it occurs in the pipe between nodes 10 and 18

Final Calculations - Hazen-Williams - 2007

RED HAWK FIRE & SECURITY
Estimate Test

Page 4
Date 012501

Hyd. Ref. Point	Qa Qt	Dia. "C" Pf/Ft	Fitting or Eqv. Ln.	Pipe Ftng's Total	Pt Pe Pf	Pt Pv Pn	***** Notes *****
1 to 2	26.00 26.0	2.157 120.0 0.0063	0.0 0.0 0.0	10.000 0.0 10.000	21.556 0.0 0.063		K Factor = 5.60 Vel = 2.28
2 to 3	26.04 52.04	2.157 120.0 0.0228	0.0 0.0 0.0	10.000 0.0 10.000	21.619 0.0 0.228		K Factor = 5.60 Vel = 4.57
3 to 4	26.17 78.21	2.157 120.0 0.0485	T 12.307 0.0 0.0	5.000 12.307 17.307	21.847 0.0 0.839		K Factor = 5.60 Vel = 6.87
	0.0 78.21				22.686		K Factor = 16.42
5 to 6	26.39 26.39	2.157 120.0 0.0065	0.0 0.0 0.0	10.000 0.0 10.000	22.215 0.0 0.065		K Factor = 5.60 Vel = 2.32
6 to 4	26.44 52.83	2.157 120.0 0.0235	T 12.307 0.0 0.0	5.000 12.307 17.307	22.280 0.0 0.406		K Factor = 5.60 Vel = 4.64
	0.0 52.83				22.686		K Factor = 11.09
7 to 8	26.03 26.03	2.157 120.0 0.0063	0.0 0.0 0.0	10.000 0.0 10.000	21.604 0.0 0.063		K Factor = 5.60 Vel = 2.29
8 to 9	26.07 52.1	2.157 120.0 0.0229	0.0 0.0 0.0	10.000 0.0 10.000	21.667 0.0 0.229		K Factor = 5.60 Vel = 4.57
9 to 10	26.20 78.3	2.157 120.0 0.0485	T 12.307 0.0 0.0	5.000 12.307 17.307	21.896 0.0 0.840		K Factor = 5.60 Vel = 6.87
	0.0 78.30				22.736		K Factor = 16.42
11 to 12	26.42 26.42	2.157 120.0 0.0065	0.0 0.0 0.0	10.000 0.0 10.000	22.264 0.0 0.065		K Factor = 5.60 Vel = 2.32
12 to 10	26.47 52.89	2.157 120.0 0.0235	T 12.307 0.0 0.0	5.000 12.307 17.307	22.329 0.0 0.407		K Factor = 5.60 Vel = 4.64
	0.0 52.89				22.736		K Factor = 11.09
15 to 16	28.57 28.57	2.157 120.0 0.0075	0.0 0.0 0.0	10.000 0.0 10.000	26.030 0.0 0.075		K Factor = 5.60 Vel = 2.51
16 to 14	28.61 57.18	2.157 120.0 0.0271	T 12.307 0.0 0.0	5.000 12.307 17.307	26.105 0.0 0.469		K Factor = 5.60 Vel = 5.02
	0.0 57.18				26.574		K Factor = 11.09
4 to 17	131.04 131.04	1.682 120.0 0.4228	T 9.9 0.0 0.0	1.000 9.900 10.900	22.686 0.0 4.608		Vel = 18.92

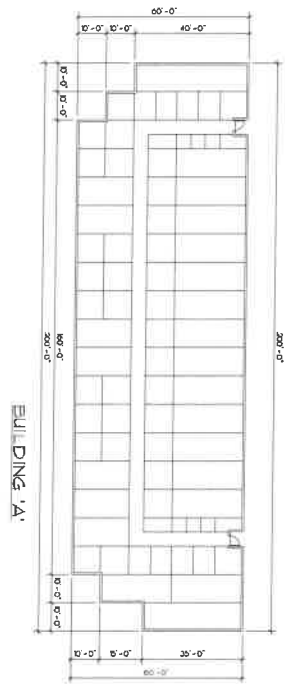
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Final Calculations - Hazen-Williams

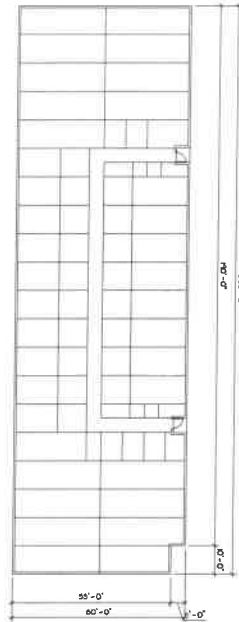
RED HAWK FIRE & SECURITY
Estimate Test

Page 5
Date 012501

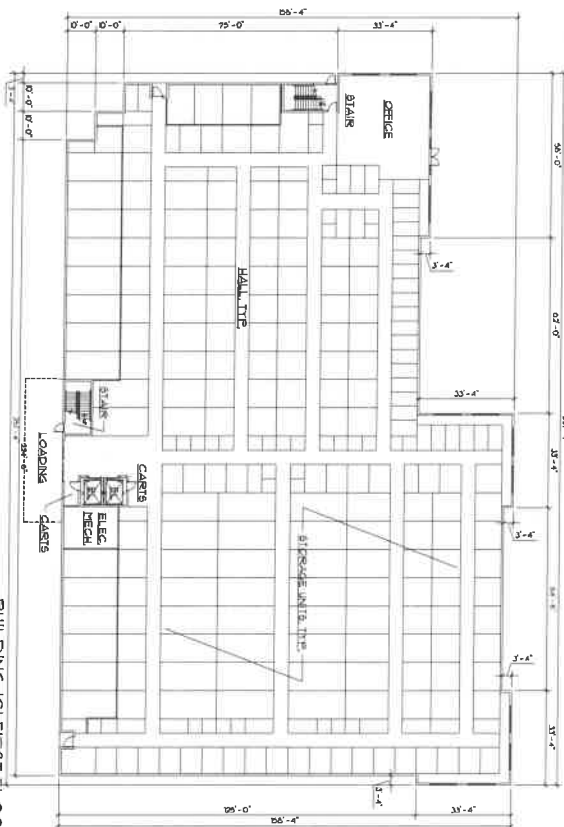
Hyd. Ref. Point	Qa Qt	Dia. "C" Pf/Ft	Fitting or Eqv.	Ln.	Pipe Ftng's Total	Pt Pe Pf	Pt Pv Pn	*****	Notes	*****
	0.0 131.04					27.294			K Factor = 25.08	
10 to 18	131.19 131.19	1.682 120.0 0.4237	T	9.9 0.0 0.0	1.000 9.900 10.900	22.736 0.0 4.618			Vel = 18.94	
	0.0 131.19					27.354			K Factor = 25.08	
14 to 19	57.18 57.18	1.682 120.0 0.0912	T	9.9 0.0 0.0	1.000 9.900 10.900	26.574 0.0 0.994			Vel = 8.26	
	0.0 57.18					27.568			K Factor = 10.89	
17 to 18	131.04 131.04	4.26 120.0 0.0046		0.0 0.0 0.0	13.000 0.0 13.000	27.294 0.0 0.060			Vel = 2.95	
18 to 19	131.19 262.23	4.26 120.0 0.0165		0.0 0.0 0.0	13.000 0.0 13.000	27.354 0.0 0.214			Vel = 5.90	
19 to 20	57.18 319.41	4.26 120.0 0.0238	T	26.334 0.0 0.0	157.000 26.334 183.334	27.568 0.0 4.362			Vel = 7.19	
	0.0 319.41					31.930			K Factor = 56.53	
20 to 21	319.41 319.41	4.26 120.0 0.0238	4E	52.668 0.0 0.0	50.000 52.668 102.668	31.930 0.0 2.443			Vel = 7.19	
21 to 22	0.0 319.41	6.357 120.0 0.0034	G C 3E	3.772 1.257 52.808	15.000 57.837 72.837	34.373 6.496 0.248			Vel = 3.23	
22 to 23	0.0 319.41	6.16 140.0 0.0030	2T Zab 5E	86.075 0.0 100.42	360.000 186.495 546.495	41.117 3.127 1.622			* Fixed Loss = 3.127 Vel = 3.44	
	250.00 569.41					45.866			Qa = 250.00 K Factor = 84.08	



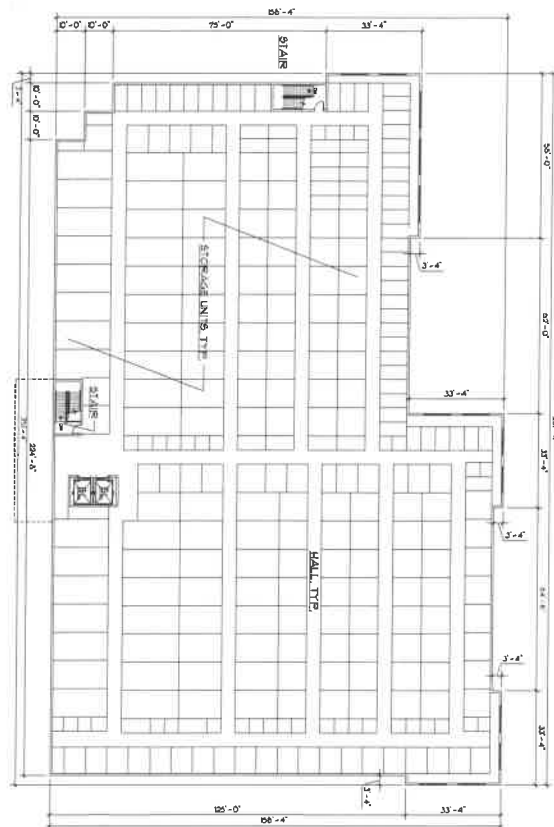
BUILDING A



BUILDING B



BUILDING C FIRST FLOOR
SCALE: 1" = 20'



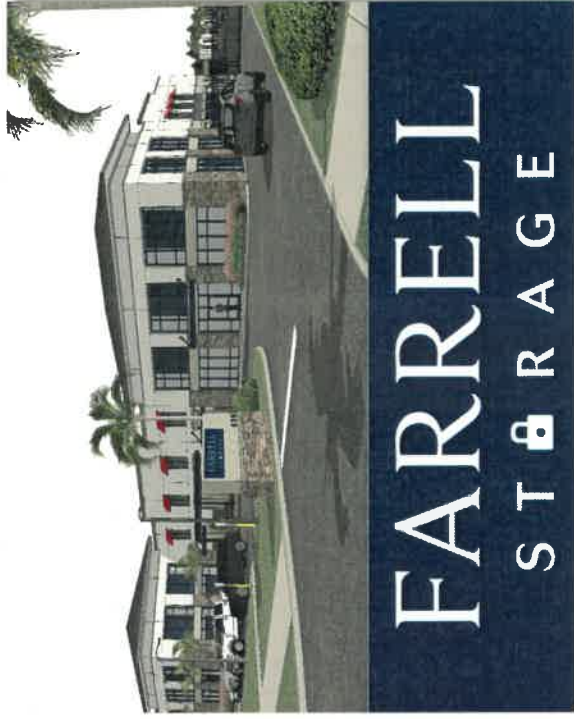
BUILDING C SECOND FLOOR

DATE: AUG. 8, 2008
JOB NO.:
FILE NO.:
SHEET
OF

A PROPOSED SELF STORAGE FACILITY
ADVANTAGE DEVELOPMENT GROUP, LLC
JENSEN BEACH, FLORIDA



**PATRICK M. PILLOT
ARCHITECT, INC.**
1267 SECOND STREET SARASOTA, FLORIDA 34236
941 866-7876 FAX: (941) 866-2381
FLORIDA 44001854 EMAIL: PATM@PILLOTARCH.COM



CONCEPTUAL DESIGN

AUGUST 23, 2018



PATRICK M. PILOT ARCHITECT, INC.

941.955.7375
1267 Second Street
Sarasota, FL 34236
pet@pilotarch.com
FL license no. AR0011554



AERIAL | VIEW FROM NORTH



PATRICK M. PILLOT ARCHITECT, INC.
 941.955.7375
 1267 Second Street
 Sarasota, FL 34236
 pat@pillotarch.com
 FL license no. AR0011554

FARRELL STORAGE

AUGUST 23, 2018

2 OF 15

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AERIAL | VIEW FROM SOUTHWEST



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 1267 Second Street
 Sarasota, FL 34236
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BUILDING C | NORTH ELEVATION



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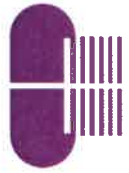
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Design features from LDC Section 4.872.C covering 50% of ground floor façade length (four required):

1. Awnings
2. Windows
3. Decorative light fixtures
4. Peaked roof forms
5. Architectural details other than those listed: trellis structures



BUILDING C | NORTHWEST CORNER



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BUILDING C | NORTHEAST CORNER



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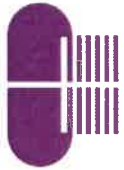
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224 SF PATIO WITH TRELLIS COVERING

GREEN SCREEN TRELLIS
OFFSET FROM WALL

BUILDING C | PEDESTRIAN PATIO DETAIL



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STUCCO FINISH

CORNICE TO MATCH
PRIMARY ELEVATION

BORAL SPEC BRIK
CHESAPEAKE BLEND OR SIMILAR

APPLIED WINDOW

BORAL CHANNEL 1x8
SW 7102 WHITE FLOUR

BUILDING A | EAST WALL DETAIL (BUILDING A WEST WALL, BUILDING B EAST WALL, AND BUILDING B WEST WALLS SIMILAR)



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SITE PLAN



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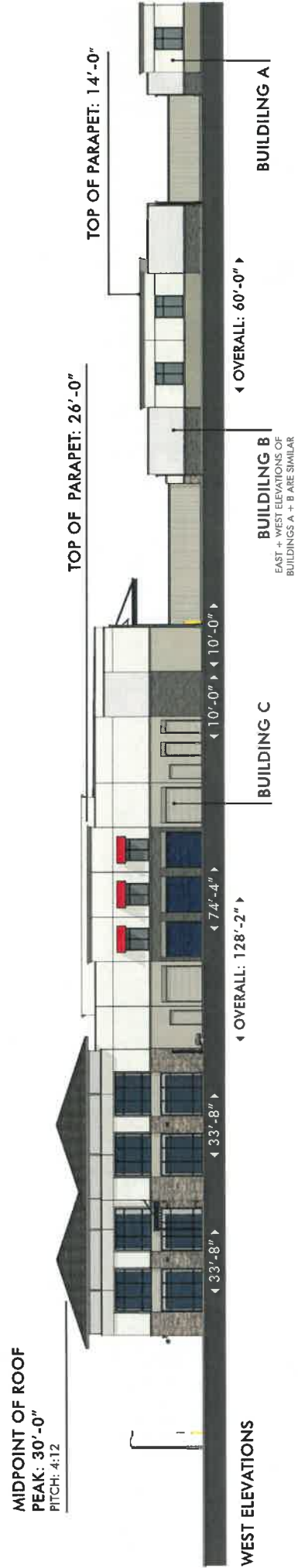
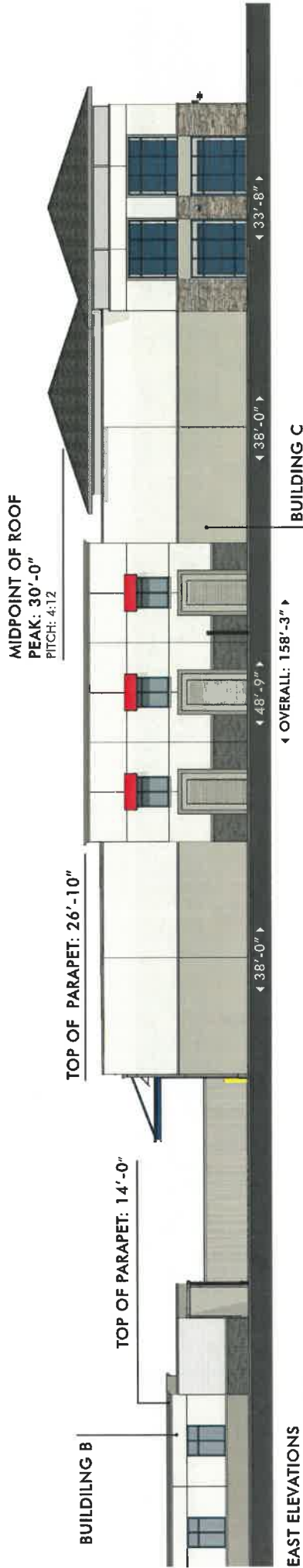
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ARCHITECTURAL ELEVATIONS



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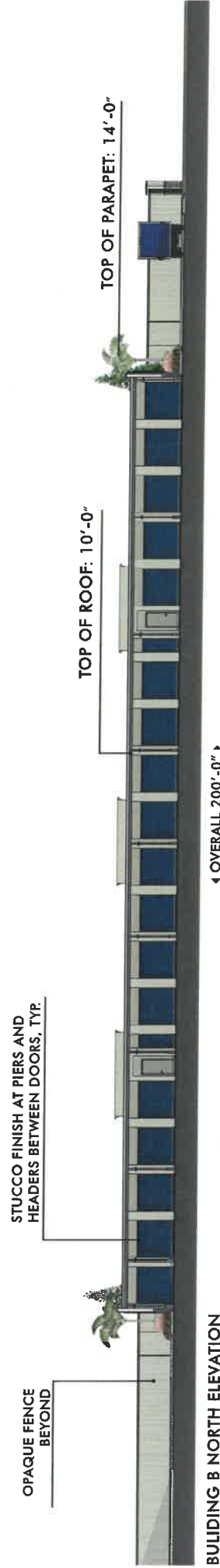
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BUILDING A SOUTH ELEVATION



BUILDING B NORTH ELEVATION
BUILDING B SOUTH ELEVATION SIMILAR
BUILDING A NORTH ELEVATION SIMILAR

ARCHITECTURAL ELEVATIONS

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A

WALL COLOR 1:
SW 7012 CREAMY

B

WALL COLOR 2:
SW 7739 HERBAL WASH

C

TRIM COLOR 3:
SW 7737 MEADOW TRAIL

D

METAL FABRICATIONS:
SW 7505 MANOR HOUSE

E

FIXED ALUM. AWNING:
SW 6864 CHERRY TOMATO

F

STOREFRONT FRAMES +
ROLL-UP DOORS:
KAYNEER PERMACOAT "DEEP BLUE"; MATCH
FOR ROLL-UP DOORS + AWNINGS

G

STONE CLADDING:
ELDORADO STONE
RUSTIC LEDGE / CASCADE

H

SIDING:
BORAL CHANNEL 1x8
SW 7102 WHITE FLOUR

I

ASPHALT ROOF SHINGLES:
GAF TIMBERLINE ULTRA HD
WEATHERED WOOD
320 POUND 30 YEAR ARCHITECTURAL
SHINGLE
MIAMI-DADE NOA 18-0123.07

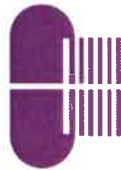
J

BRICK CLADDING:
BORAL SPEC BRK
CHESAPEAKE BLEND

K

TRELLIS:
GREEN SCREEN WALL MOUNT-
ED TRELLIS PANEL OR SIMILAR

COLOR & MATERIALS PALETTE



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COLOR & MATERIALS KEY



WALL COLOR 1:
SW 7012 CREAMY



WALL COLOR 2:
SW 7739 HERBAL WASH



TRIM COLOR 3:
SW 7737 MEADOW TRAIL



METAL FABRICATIONS:
SW 7505 MANOR HOUSE



FIXED ALUM. AWNING:
SW 6864 CHERRY TOMATO



**STOREFRONT FRAMES +
ROLL-UP DOORS:**
KAWNEER PERMACOAT "DEEP BLUE", MATCH
FOR ROLL-UP DOORS + AWNINGS



STONE CLADDING:
ELDORADO STONE
RUSTIC LEDGE / CASCADE



SIDING:
BORL CHANNEL 1x8
SW 7102 WHITE FLOUR



ASPHALT ROOF SHINGLES:
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BRICK CLADDING:
BORL SPEC BRIK
CHESAPEAKE BLEND



TRELLIS:
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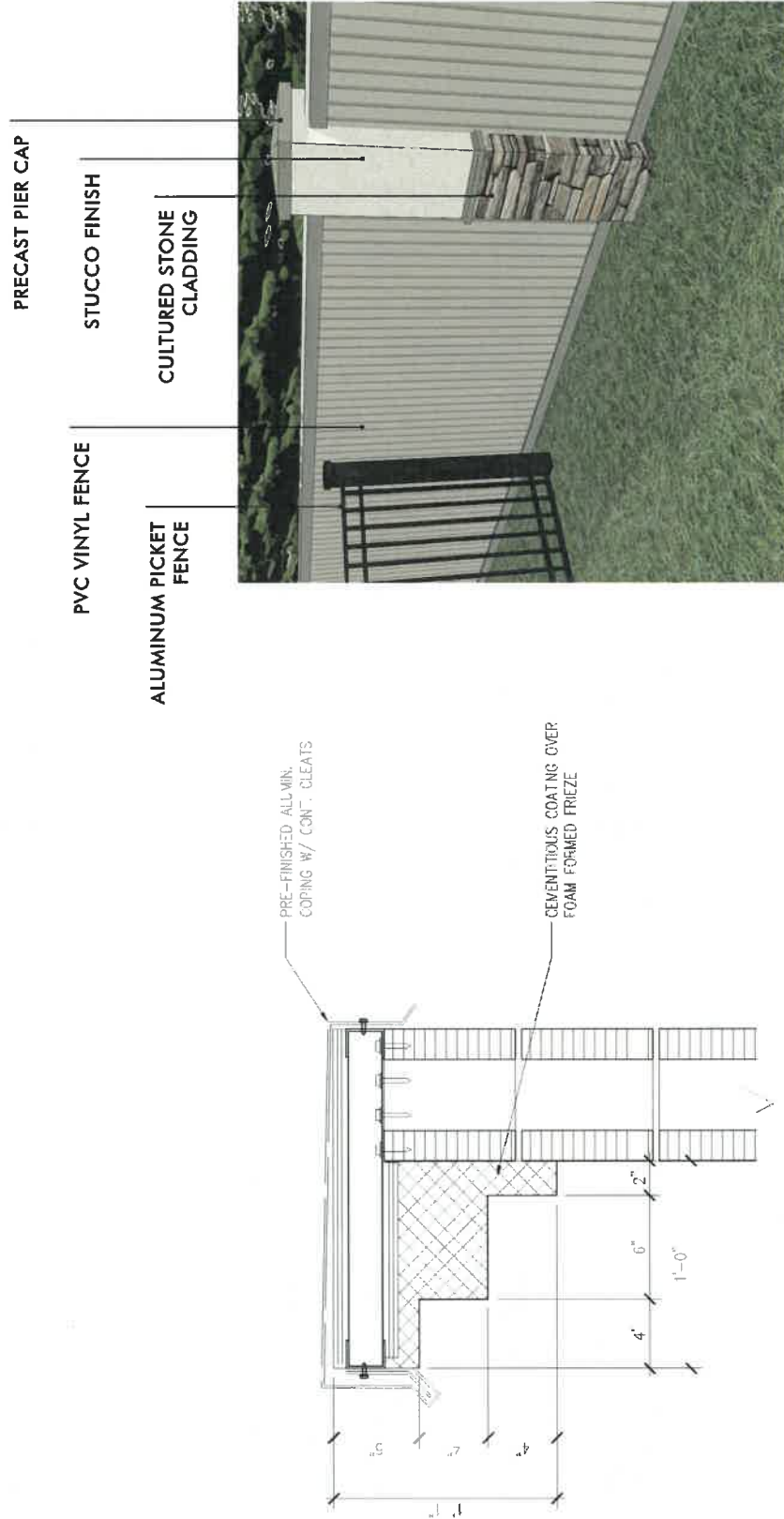
COLOR & MATERIALS KEY



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CORNICE DETAIL

PERIMETER OPAQUE FENCE DETAIL

DETAILS



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	S.F.	Acres	% of Site
Total Site Area	193,400	4.44	100.00%
Upland Preserves Provided	45,469	1.04	23.5%
Total Development Area	147,931	3.4	76.5%
- Wetland Mitigation: 20% of Development Area (1 of 17.7 acres)	29,586	.68	20%
- Development Area Trees Required (12,500 S.F. = 1 acre)	11,677	.27	59 Trees
- Existing Palms To Remain (16 Palm20 = 5 Trees)			5 Trees
- Palms Provided (99 Palm20 = 23 Trees)			23 Trees
- Trees Provided			123 Trees
Total Trees Provided			
- Upland Preserves Provided	39,855	.9	20.6%
- Perimeter Buffer Areas	17,374	.4	9.0%
- Interior Areas	1,074	.03	0.6%
Sub-Total	48,029	1.1	24.9%
Interference Area			
- Vehicular Use Area	40,806	.92	21.1%
- Building Area	50,000	1.13	26.2%
- Sidewalks & Concrete, Paved, Etc.	1,203	0.03	0.6%
Sub-Total	99,889	2.29	51.5%
Vehicular Use Area Buffer: 10' depth left/right/10' = Hedge			
- Area Required (300' x 1' / 5,000 S.F. = paved area)			4,008 S.F.
- Area Required			4,008 S.F.
- Trees Required			2 (9' Trees or 3 (7' Trees)
- VUA, Trees Provided			15 Sabal, 12 (2' Cyp)
- VUA, Trees Required			15 Sabal, 12 (2' Cyp)

[illegible]

Drawn By	DMS
Checked By	MM
Scale	1" = 30'
Drawing Date	08/30/2018

5

Scale: 1" = 30'-0"

975

[illegible][illegible]

Fig.	Scientific Name	Common Name	Size	Spawning
70	<i>Scorpaenopsis</i>	Sold fish	10", 14", 18" CL, 8000000 length	AS
71	<i>Scorpaenopsis</i>	Porcupine fish	10" TL, 6000000 length	AS
72	<i>Scorpaenopsis</i>	Porcupine fish	10" CL, 1000000 length	AS

[illegible][illegible][illegible]

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 • SEE INSTRUCTIONS FOR SUCCESSFUL REGISTRATION

1010171146 QRP Antenna Theories and Applications 2009 •

[illegible]

Tree #	Species	Dbd (cm survey)	Status	Height (m)	Time #	Species
504	PALM	25"	non-surv		6040	PALM
506	PALM	12"	remains		6041	PALM

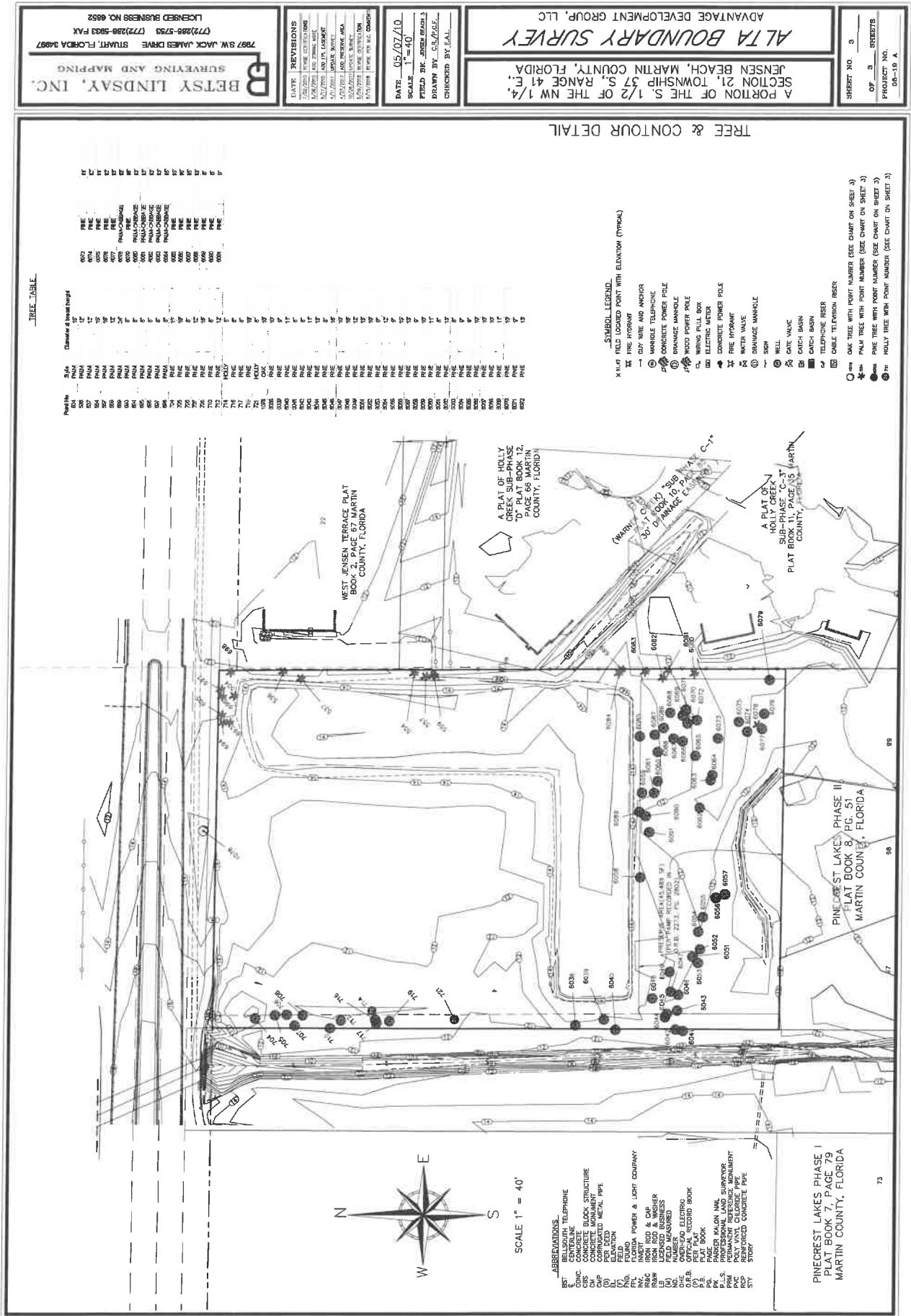
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539	PA1M	12"	PA1E
540	PA1M	12"	PA1E
541	PA1M	12"	PA1E
542	PA1M	12"	PA1E
543	PA1M	12"	PA1E
544	PA1M	12"	PA1E
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Prior to the installation of any pipe, material or existing and proposed wells shall be tested and approved to meet the well specifications and spacing plans needed as shown on this plan.

60250	NAME	5	NAME	6060
60250	NAME	5	NAME	6060

Drawn By _____ DMS
Checked By _____ MM
Scale _____
Drawing Date 08/30/2018

2.



DISCLOSURE OF INTEREST AFFIDAVIT

BEFORE ME, the undersigned authority, duly authorized to take acknowledgments and administer oaths, personally appeared the undersigned person on the date set forth below, who, first being duly sworn, deposes and says under penalties of perjury:

1. That the record property owner(s) of the Real Property described in **Exhibit "A"** to this Affidavit is (are) as follows:

Name	Address
Jensen Beach Holdings, LLC.	2049 SW Poma Drive, Palm City, FL. 34990

(If more space is needed attach separate sheet)

2. That the following is a list of every natural person and entity with any legal or equitable interest in the property (as defined in Section 10.2.B.3. Land Development Regulations, Martin County Code):

Name	Address	Interest
Frank Poma	12212 Riverbend Court Port St. Lucie, FL. 34984	Partner
Kimberly D. Poma	12212 Riverbend Court Port St. Lucie, FL. 34984	Partner
Jason F. Poma	4537 SW Long Bay Drive Palm City, FL. 34990	Partner

(If more space is needed attach separate sheet)

3. That the following is a list of those, who have any interest in a contract for sale of the property, or a conveyance of any interest in the property, including but not limited to, real estate brokers and salespersons; and any and all mortgagees of the property:

Name	Address	Interest
Advantage Development Group, LLC.	P.O. Box 7324 Golden, CO 80403	See Attached Purchaser Applicant
Jensen Beach Holdings, LLC.	2049 SW Poma Drive, Palm City, FL. 34990	Owner Seller See Attached

(If more space is needed attach separate sheet)

4. That the following is a list of all other applications for which the applicant has an interest as defined in subsection b. and c. of Section 10.2.B.3. Land Development Regulations, Martin County Code currently pending before Martin County. The list shall include any development applications, waiver applications, road opening applications, and lien reduction requests.

Application Name and/or Project Number	Names & Addresses of Parties involved	Date	Type of Application	Status of Application*
None				

(If more space is needed attach separate sheet)

- Status defined as:
A = Approved

P = Pending
D = Denied
W = Withdrawn

This Affidavit is given for the purpose of establishing compliance with the provisions of Section 10.2.B.3 Land Development Regulations; Martin County Code.

FURTHER AFFIANT SAYETH NOT.

AFFIANT

STATE OF Colorado
COUNTY OF Jefferson

The foregoing Disclosure of Interest Affidavit was sworn to, affirmed and subscribed before me this 19 day of January 2018, by Jeffrey S. Kinder, who is personally known to me or have produced Colorado Driver License as identification.

Notary Public, State of Colorado
Print Name: Mark Stazzone
My Commission Expires: 6/7/2021

(Notary Seal)

MARK STAZZONE
NOTARY PUBLIC
STATE OF COLORADO
NOTARY ID 20174024040
MY COMMISSION EXPIRES JUNE 7, 2021

Attachment
To
Disclosure of Interest Affidavit

Advantage Development Group, LLC
P.O. Box 7324
Golden, CO 80403

Parties of Interest

Advantage Advisors, LLC, Member
34628 Lyttle Dowdle Dr.
Golden, CO 80403

James C. Wojcik Living Trust, Member
87 Laurel Hill Rd.
Northport, NY 11768

Stephen H. Zagoren, Member
9 Woodpath Dr.
Northport, NY 11768

EXHIBIT A

Advantage Self Storage – Jensen Beach
528 NE Jensen Beach Boulevard 5, Jensen Beach, FL
Parcel ID: 21-37-41-000-000-00244-8
(4.4 acres – Parcel 1 - 4.26 acres; Parcel 2 – 0.18 acres)

Legal Description

PARCEL 1:

THE EAST 337.66 FEET OF THE SOUTH ONE-HALF OF THE NORTHWEST ONE-QUARTER OF SECTION 21, TOWNSHIP 37 SOUTH, RANGE 41 EAST, LYING AND BEING IN MARTIN COUNTY, FLORIDA. LESS AND EXCEPT ALL OF PINECREST LAKES PHASE II AND LESS AND EXCEPT ROAD RIGHT OF WAY FOR JENSEN BEACH BOULEVARD.

PARCEL 2:

A PARCEL LAND BEING A PORTION OF THE SOUTH 1/2 OF SECTION 21 OF LAND BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS: BEGINNING AT INTERSECTION OF THE WEST LINE OF THE EAST 337.66 FEET OF THE SOUTH 1/2 OF THE NORTHWEST 1/4 OF SAID SECTION 21 WITH THE SOUTH RIGHT OF LINE OF JENSEN BEACH BOULEVARD, BEING A 100 FOOT RIGHT OF WAY; THENCE S00°09'39"E ALONG THE WEST LINE OF SAID EAST 337.66 FEET A DISTANCE OF 550.27 FEET TO AN INTERSECTION WITH THE NORTH LINE OF PINE CREST LAKES II, AS RECORDED IN PLAT BOOK 8, PAGE 51 OF THE PUBLIC RECORDS OF MARTIN COUNTY, FLORIDA; THENCE N89°31'14"W ALONG SAID NORTH LINE A DISTANCE OF 15.66 FEET TO AN INTERSECTION WITH THE EAST LINE OF A DRAINAGE RIGHT OF WAY PER OFFICIAL RECORDS BOOK 221, AT PAGE 240 OF THE PUBLIC RECORDS OF MARTIN COUNTY, FLORIDA; THENCE N00°10'39"E ALONG SAID EAST LINE A DISTANCE OF 550.24 FEET TO AN INTERSECTION WITH SAID SOUTH RIGHT OF WAY LINE OF JENSEN BEACH BOULEVARD; THENCE S89°31'38"E ALONG SAID SOUTH RIGHT OF WAY LINE A DISTANCE OF 12.41 FEET TO THE POINT OF COMMENCEMENT. SAID PARCEL CONTAINING 0.18 ACRES MORE OR LESS.

Prepared By:



Engineering & Planning, Inc.

Dated: August 24, 2018

Appendix
Article 10.2.B.3. Article 10, Development Review Procedures;
Land Development Regulations; Martin County Code

10.2.B. Application submittal for development approval. Applications for development approval shall comply with the following described procedures:

1. Initiation. A development application shall be filed with the County Administrator by the owner or other person having a power of attorney from the owner to make the application.
2. Acceptance of the application. A development application will be received for processing on any working day.
3. Verification of property ownership. The documents required below are required prior to an application being determined complete. After the application is determined to be complete, the applicant has a continuing obligation to provide revised documents to reflect any changes to the information provided that may occur before and as of the date of the final public hearing or final action on the application.
 - a. Proof of ownership must be provided for any application for any type of development order. The applicant shall provide a copy of the recorded deed for the subject property, and shall certify any subsequent transfers of interests in the property. If the applicant is not the owner of record, the applicant is required to report its interest in the subject property.
 - b. The applicant must disclose the names and addresses of each and every natural person or entity with any legal or equitable interest in the property of the proposed development, including all individuals, children, firms, associations, joint adventures, partnerships, estates, trusts, business trusts, syndicates, fiduciaries, corporations, limited liability company, professional associations and all other groups or combinations.
 - c. For those entities that are a firm, association, joint adventure, partnership, estate, trust, business trust, syndicate, fiduciary, corporation, limited liability company, professional associations and all other groups or combinations thereof, every natural person or entity that enjoys a legal or equitable interest in property of the proposed development shall be disclosed including but not limited to any partners, members, shareholders, trustees, and stockholders.
 - d. The disclosure required in b. and c. above shall not apply to companies that are publicly traded and to consultants and contractors who may perform professional services or work related to the property.
 - e. In addition, the disclosure must include those having any interest in a contract for sale of the property, or a conveyance of any interest in the property, including but not limited to, real estate brokers and salespersons; and any and all mortgagees of the property.
 - f. The applicant must list all other applications for which they have an interest as defined in subsection b. and c. above that is currently pending before Martin County. The list shall include any development applications, waiver applications, road opening applications, and lien reduction requests.
 - g. Any development order, including applications for Planned Unit Developments which was granted or approved based on false or incomplete disclosure will be presumed to have been fraudulently induced and will be deemed by the Martin County Board of County Commissioners to be void ab initio and set aside, repealed, or vacated.



Engineering & Planning, Inc.

1172 SW 30th Street • Suite 500 • Palm City • Florida • 34990

(772) 286-8030 • www.MackenzieEngineeringInc.com

August 24, 2018

Mr. Paul Schilling
Martin County Growth Management
2401 SE Monterey Road,
Stuart, FL, 34996

Re: Advantage Development Group
Advantage Self Storage – Revised Major Final Site Plan
Parking Rate Adjustment Analysis

Shaun G. MacKenzie P.E.
Florida License No. 61751
Transportation Engineer
Engineering Business No. 29013

Dear Mr. Schilling,

Mackenzie Engineering and Planning, Inc. prepared this Parking Rate Adjustment analysis for the proposed project, Advantage Self Storage. The proposed project is located at 528 NE Jensen Beach Blvd, between SE Green River Parkway and NE Savannah Road, Jensen Beach, FL 34957 (Parcel ID: 21-37-41-000-000-00244-8). The proposed project is for the construction of a 92,700 SF storage facility.

Parking Generation

The proposed project utilized the trip generation rates published by the *Institute of Transportation Engineers' (ITE) Report, Parking Generation (4th Edition)*. The proposed site was evaluated using the trip generation rate for Mini Warehouse/SS (Land Use 151) as shown in the Appendix. Table 1 presents the proposed project's parking generation.

Table 1. Parking Generation

ITE Parking Rate	Proposed SF (X) per 1,000 GFA	Required Parking per ITE Parking Rate	Provided Parking
$P = 0.07(X) + 4$	92.700	10.5	11

Parking Use & Access

The proposed development of this use typically requires four to six parking spaces in front of the building. The rear buildings are all accessed from the exterior, where tenants typically access the units by parking in front of their unit for a short period of time. The 24' driveway aisles between the buildings support this activity. Additionally, the site provides four (4) loading spaces along the rear of Building 3 per Martin County Code (see attached Site Plan).

Parking Supply

The site consists of 10 standard parking, 1 handicap parking, and 4 loading spaces (see attached Site Plan).

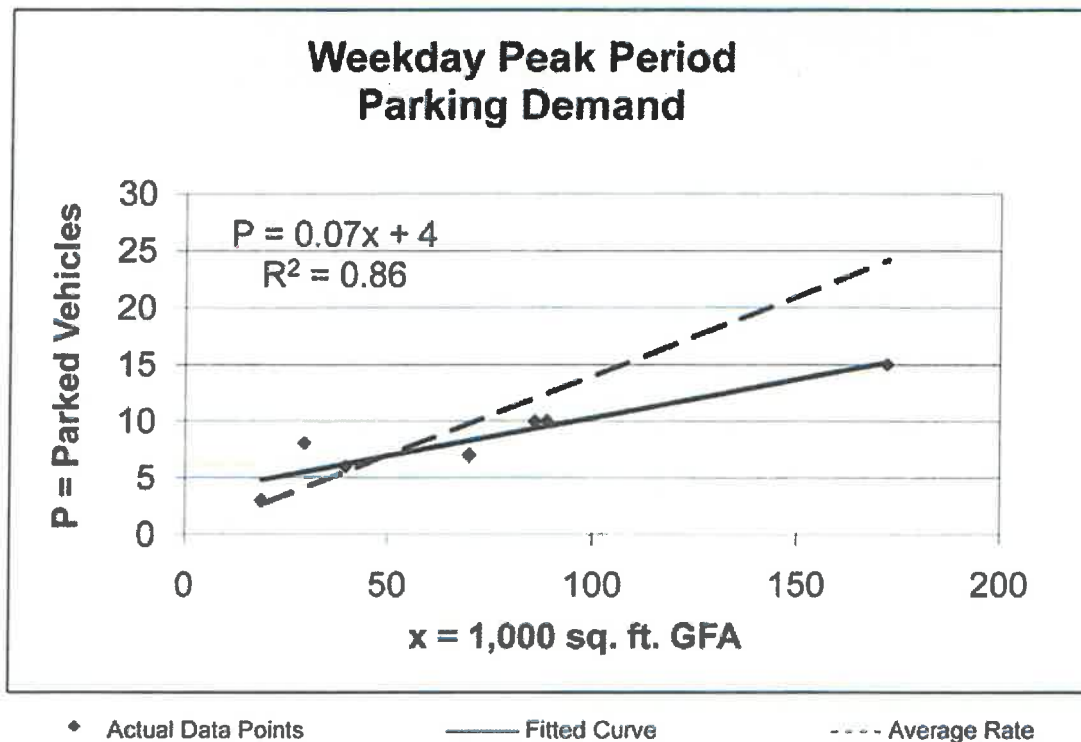
Recommendation

Based on ITE's Report, Parking Generation (4th Edition), the site has a required parking of 11 spaces. The site has a supply of 11 parking spaces. The proposed rate adjustment will not result in undesirable overflow parking, nor otherwise adversely impact the character and integrity of the surrounding area. Therefore, the site is projected to have an adequate supply of parking based on ITE's Report, Parking Generation (4th Edition).

Land Use: 151 Mini-Warehouse

Average Peak Period Parking Demand vs. 1,000 sq. ft. GFA On a Weekday

Statistic	Peak Period Demand
Peak Period	10:00 a.m.–12:00 p.m.; 4:00–5:00 p.m.
Number of Study Sites	7
Average Size of Study Sites	72,000 sq. ft. GFA
Average Peak Period Parking Demand	0.14 vehicles per 1,000 sq. ft. GFA
Standard Deviation	0.06
Coefficient of Variation	44%
Range	0.09–0.27 vehicles per 1,000 sq. ft. GFA
85th Percentile	0.17 vehicles per 1,000 sq. ft. GFA
33rd Percentile	0.11 vehicles per 1,000 sq. ft. GFA



SITE DATA TABLE	
TOTAL SITE AREA	4.41 AC (191,806 SF)
ZONING	COB-1
FUTURE LAND USE	COMMERCIAL OFFICE AND RESIDENTIAL

[illegible]

SURFACE WATER	BUILDING DATA	94C
WATER ALLOWED	PROVIDED	
MAN COVERAGE 40%	90%	
MANIFEST 20%	70%	
NUMBER OF BUILDINGS	3	
NUMBER OF STORIES	(2 FLOORS, 210 STORIES)	
WATER PROVISION	MARTIN COUNTY UTILITIES DEPT	
SEWER PROVISION	MARTIN COUNTY UTILITIES DEPT	
ELECTRICITY PROVIDED	FLORIDA POWER & LIGHT	

[illegible]

Requirement	Standard	Minimum
STANDARD PARKING REQUIRED	11	11
STANDARD PARKING SUPPLY	10	10
REQUIRED HANDICAP PARKING	1	1
HANDICAP PARKING PROVIDED	1	1
REQUIRED LOADING SPACES	4	4
PROVIDED LOADING SPACES	4	4
TOTAL PARKING PROVIDED	11	11
WHEELCHAIR ACCESSIBLE SPACES PROVIDED	2	2

[illegible]

The diagram shows two cross-sections of a sidewalk. The left section, labeled 'HANDICAP SPACE DETAIL', shows a 20'0" wide area with a 18'0" clear width. It includes a 5'0" high curb, a 2'0" high curb, and a 2'0" high curb. The right section, labeled 'REGULAR SPACE DETAIL', shows a 20'0" wide area with a 18'0" clear width. It includes a 2'0" high curb and a 2'0" high curb. Both sections show a 2'0" high curb and a 2'0" high curb.

PARCELS 2-A PARCEL LAND BEING A PORTION OF THE SOUTH 1/2 OF SECTION 28 TOWNSHIP 6 NORTH RANGE 7 EAST ALONG SUND NORTHLAND DISTANCE OF 15-00 FEET TO AN INTERSECTION WITH THE SOUTH RIGHT OF WAY LINE OF KENSIN BEACH BOULEVARD; THENCE S89°21'W E, ALONGSAYE LANE AND WESTERN AVENUE TO THE CITY CENTER TRAILER PARK; ALL ARE BEING SUBJECT TO ANY/all EASEMENTS, RESERVATION, RESTRICTIONS OR DEDICATIONS.

38

36

11

PLAN
LLC
A

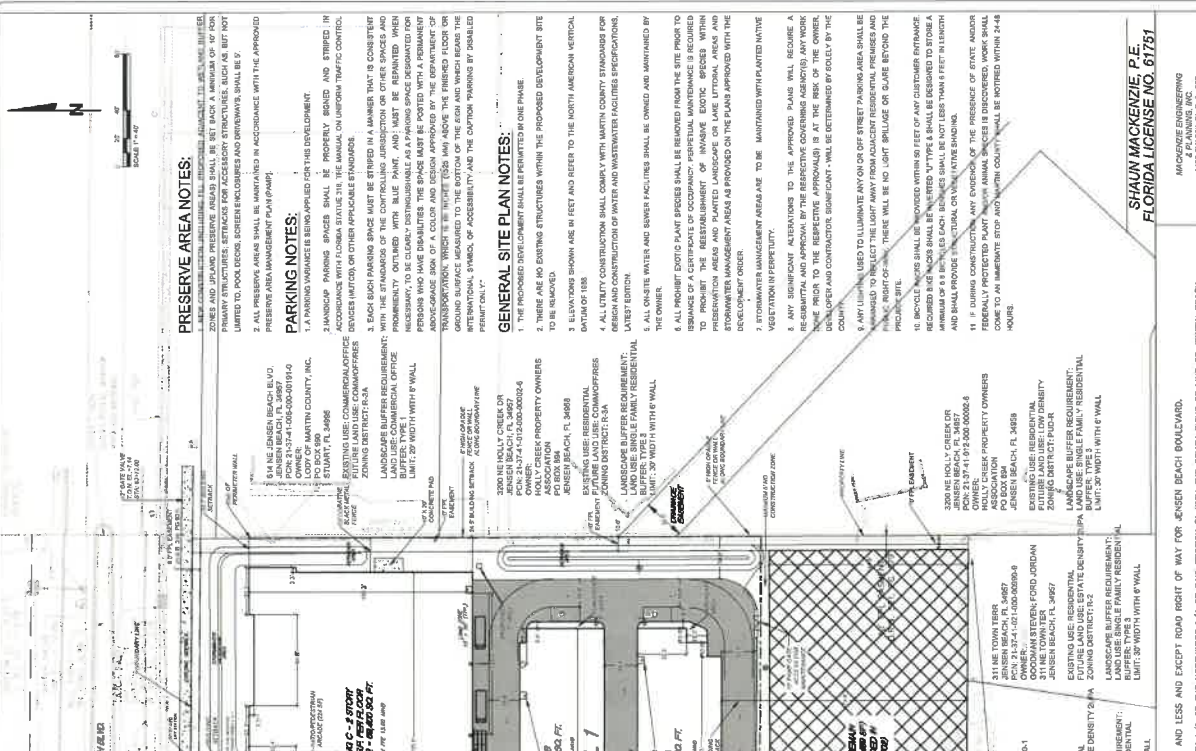
REVISION
ADVANTAGE
JEN

SCALE 1" = 40'
APPROVED: SM
DRAWN: EJJ
CHECKED: SGW

DATE: 08/24/2018	COUNTY PROJ #:
FIELD BOOK NO:	J040 - 011
	C RECORD #:
	DEV2018040004
	DESIGNER:
	EN

DATE:	SGM
SUBMITTAL:	
08/24/2018	
REVISION:	
1. MC 08/13/2018	

MEP PROJECT
124-001
COA NO.
29013



PRENDE MOOTD39'E ALONG S40 EAST LINE A DISTANCE OF 550.24 FEET TO AN INTERSECTION WITH S40 E55S.	1772 1/2 S. PALM BLVD. APT. 500 PALM CITY, FLORIDA 34909 772-280-7700 C48902
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Engineering & Planning, Inc.

1172 SW 30th Street • Suite 500 • Palm City • Florida • 34990

(772) 286-8030 • www.MackenzieEngineeringInc.com

August 24, 2018

Mr. Doug Killane, Bureau Chief Fire Prevention
Fire Rescue
Martin County Growth Management Department
2401 SE Monterey Road
Stuart FL, 34996

Re: Advantage Development Group
Advantage Self Storage – Revised Major Final Site Plan
Project Number: J040-011
Fire Prevention Compliance Letter

Dear Mr. Killane:

On behalf of Advantage Development Group, I certify that the proposed project will be in compliance with Fire Prevention Code Regulations. The site is currently an undeveloped commercial property located at 528 NE Jensen Beach Blvd, between SE Green River Parkway and NE Savannah Road, Jensen Beach, FL 34957 (Parcel ID: 21-37-41-000-000-00244-80000, 4.44 Acres). The proposed project is for the construction of a 92,700 SF storage facility.

The storage facility will consists of the following:

- Fire hydrant located in front of Building C (located within 500 LF of the furthest building).
- Fire sprinklers will be fed by a 6" fire line with a single 6" backflow preventer.
- Fire Alarm Installation.
- Fire Retardant construction material.

If you have any questions, please do not hesitate to contact Shaun MacKenzie @ (772) – 286-8030 or shaun@mackenzieengineeringinc.com.

Shaun G. MacKenzie P.E.
Florida License No. 61751

124001
August 2018
© MacKenzie Engineering and Planning, Inc.
CA 29013

Engineering & Planning, Inc.

1172 SW 30th Street, Suite 500 • Palm City • Florida • 34990

(772) 286-8030 • www.mackenzieengineeringinc.com

December 14, 2018

Catherine Riiska, MSM PWS, Principal Planner
Martin County Growth Management Department
2401 SE Monterey Road
Stuart, FL 34996
Phone: 772-288-5667
Email: criiska@martin.fl.us

Re: **Advantage Self Storage – Jensen Beach, FL**
Revised Major Final Site Plan
MC Project No.: J040-011
MC Record No.: DEV2018040004

Dear Catherine:

We offer the following responses to comments dated **October 31, 2018** regarding the Martin County Growth Management Department (MCGMD) staff's review of the Major Final Site Plan Application.

The response to comments were provided by the following:

- MacKenzie Engineering & Planning, Inc. (**MEP**) – Civil Engineer
- Thomas Engineering Group, Inc. (**TEG**) – Drainage Engineer

Per request, we've included the following documents in our submittal package:

- A.0. Parking Rate Adjustment Analysis
- A.1. Pinecrest Phase 1 Permit
- A.2. Pinecrest Phase 2 Permit
- A.3. Pinecrest Phase 10 Calculations
- A.4. Holly Creek Permit
- A.5. Geotechnical Report

If you have any questions, please do not hesitate to contact Shaun Mackenzie at (772) 834-8909 or shaun@mackenzieengineeringinc.com.

Sincerely,



Shaun G. MacKenzie, P.E.
Transportation Engineer
Florida Registration Number 61751
Engineering Business Number 29013



MARTIN COUNTY, FLORIDA DEVELOPMENT REVIEW

STAFF REPORT

A. Application Information

ADVANTAGE SELF STORAGE JENSEN BEACH Revised Major Final Site Plan

Applicant:	Jensen Beach Holdings, LLC
Property Owner:	Jensen Beach Holdings, LLC
Agent for the Applicant:	McCarthy Summers Bobko et al PA, Terence P. McCarthy
County Project Coordinator:	Catherine Riiska, MS, PWS, Principal Planner
Growth Management Director:	Nicki van Vonno, AICP
Project Number:	J040-011
Record Number:	DEV2018040004
Report Number:	2018_1031_J040-011_DRT_Staff_FINAL.docx
Application Received:	04/10/2018
Transmitted:	04/11/2018
Staff Report:	06/13/2018
Workshop Meeting:	07/05/2018
Resubmittal Received:	09/10/2018
Transmitted:	09/12/2018
Staff Report:	10/31/2018
Workshop Meeting:	11/15/2018

B. Project description and analysis

This application is a request by 5051, LLC, for approval of a revised major final site plan for a residential storage facility project on two parcels totaling approximately 4.4 acres located at 528 NE Jensen Beach Boulevard in Jensen Beach, on the south side of NE Jensen Beach Boulevard approximately 700 feet east of NE Pinecrest Lakes Boulevard. This site was originally approved for a commercial project known as the Jensen Beach Professional Center, which did not proceed. The currently proposed project consists of one (1) two-story climate controlled building adjacent to Jensen Beach Boulevard and two (2) one-story buildings in the rear of the property for a total of 93,900 square feet of rentable space containing approximately 850 residential storage units. Included in this application is a request for a Certificate of Public Facilities Reservation.

The subject site consists of two parcels and has a future land use designation of Commercial Office/Residential (COR) and is zoned COR-1, Commercial Office/Residential District. In addition to the zoning standards, the application will be required to comply with the commercial design standards, landscape buffering requirements for commercial development adjacent to residential uses, and applicable Comprehensive Plan requirements for the COR future land use designation. Access is proposed via NE Jensen Beach Boulevard and the applicant has proposed a reduced parking rate and shall provide a parking rate adjustment analysis for consideration. The project is located within the

Primary Urban Services District and will be serviced by Martin County Utilities for water and wastewater services.

C. Staff recommendation

The specific findings and conclusion of each review agency related to this request are identified in Sections F through T of this report. The current review status for each agency is as follows:

Section	Division or Department	Reviewer	Phone	Assessment
F	Comprehensive Plan	Catherine Riiska	288-5667	Non-Comply
F	ARDP	Samantha Lovelady	288-5664	N/A
G	Development Review	Catherine Riiska	288-5667	Non-Comply
H	Urban Design	Santiago Abasolo	288-5485	Comply
H	Community Redevelopment	Santiago Abasolo	288-5485	N/A
I	Property Management	Colleen Holmes	288-5794	N/A
J	Environmental	Shawn McCarthy	288-5508	Comply
J	Landscaping	Karen Sjolholm	288-5909	Comply
K	Transportation	Lukas Lambert	221-2300	Comply
L	County Surveyor	Tom Walker	288-5928	N/A
M	Engineering	David Moore	320-3057	Non-Comply
N	Addressing	Emily Kohler	288-5692	Comply
N	Electronic File Submission	Emily Kohler	288-5692	Comply
O	Water and Wastewater	James Christ	320-3034	Comply
O	Wellfields	James Christ	320-3034	Comply
P	Fire Prevention	Doug Killane	288-5633	Comply
P	Emergency Management	Dan Wouters	219-4942	N/A
Q	ADA	Kevin Landry	320-3046	Comply
R	Health Department	Todd Reinhold	221-4090	N/A
R	School Board	Kimberly Everman	223-3105	N/A
S	County Attorney	Krista Storey	288-5443	Review Ongoing
T	Adequate Public Facilities	Catherine Riiska	288-5667	Review Pending

D. Review Board action

This application meets the threshold criteria for a major development, pursuant to Section 10.11.B., LDR, Martin County, Fla. (2016), and requires two public hearings. The two hearings will provide the public an opportunity to participate in the review and decision making process.

The first public hearing shall be before the Local Planning Agency, who will make a recommendation on the request, pursuant to Section 10.4., LDR, Martin County, Fla. (2016).

The second public hearing shall be before the Board of County Commissioners, who will take final action on the request, pursuant to Section 10.5., LDR, Martin County, Fla. (2016).

Pursuant to Section 10.1.F, LDR, Martin County, Fla. (2016), it shall at all times be the applicant's responsibility to demonstrate compliance with the Comprehensive Growth Management Plan (CGMP), LDR, and the Code.

E. Location and site information

Parcel number(s) and address:

21-37-41-000-000-0024.4-8

Existing Zoning:

Future land use:

Gross area of site:

528 NE Jensen Beach Blvd

COR-1, Commercial Office/Residential

FLU-COR, Future Land Use Commercial Office-Res

4.4 acres

Figure 1: Location Map



Figure 2: Subject Site 2017 Aerial with Project Linework



Adjacent existing or proposed development:

To the north: Savannah State Park (across NE Jensen Beach Blvd)
To the south: Single Family Residential
To the east: Commercial, Retail, and Multifamily Residential
To the west: Single Family Residential (across Drainage ROW)

Figure 3: Local Area 2017 Aerial with Preserve Areas



Zoning district designations of abutting properties:

To the north: R-3A, Liberal Multiple Family (across NE Jensen Beach Blvd)
To the south: R-2, Single Family Residential
To the east: R-3A, Liberal Multiple Family, and PUD-R
To the west: RS-6, Single Family Residential (across Drainage ROW)

Figure 4: Zoning Map



To the north: Conservation (across NE Jensen Beach Blvd)
 To the south: Estate Density 2 UPA
 To the east: COR, Commercial Office/Residential
 To the west: Medium Density Residential (across Drainage ROW)

MEP RESPONSE: Acknowledged.

2. As previously requested, please clarify the limits of required landscape buffers by graphically showing them on the project site with dimensions and labels. Staff cannot distinguish between the proposed landscape buffers that are required by code and pervious areas adjacent to existing commercial and residential development that are not proposed to meet the requirements of a landscape buffer on the site plan.

MEP RESPONSE: Acknowledged. Please see revised plans.

3. As previously requested, please revise the legend to include only those items that are shown on the site plan (e.g. truncated dome mats are in the legend but not shown on the site plan).

MEP RESPONSE: Acknowledged. Please see revised plans.

4. Please revise the parking data as requested in Item #2 below.

MEP RESPONSE: Acknowledged. Please see revised plans.

5. Please revise the structure setback data within the Building Data table to reflect the minimum required structure setbacks per the COR-1 zoning standards in Table 3.12.2., Section 3.12., LDR, Martin County, Fla. (2013). This table should only include codified zoning standards and a demonstration of compliance with the Article 3 standards.
 - a. Revise the header from “Max Allowed” to “Required” since standards include both maximum and minimum values.
 - b. Remove all references to land uses, landscaping buffers, VUA, and walls from the site data table.
 - c. Remove references to water, sewer and electricity providers from the site data table.
 - d. Structure setback values must be listed as “Min” (minimum) and consist of linear feet only. Required structure setbacks should be labeled as follows in the table:
 - i. Front (North) 25’ Min.
 - ii. Side (East and West) 10’ Min.
 - iii. Rear (South) 20’ Min.

MEP RESPONSE: Acknowledged. Please see revised plans.

Item #2:

Parking Rate Adjustment

Pursuant to Section 4.624., LDR, Martin County, Fla. (2009), the parking rate for Warehouse, mini, residential storage facility is 1 space per 1,500 s.f. gross floor area. The applicant has provided an alternative rate on the site plan that is inconsistent with the established parking rates for the proposed use.

Remedy/Suggestion/Clarification:

1. Please revise the parking calculations to identify the number of parking and loading spaces “Required” by the Martin County code pursuant to Table 4.14.1, Section 4.624., LDR, Martin County, Fla. (2009), which identifies the required rate for residential storage facilities to be 1 space per 1,500 square feet of gross floor area. The “Provided” number would then be footnoted as “per parking rate adjustment.”
2. The provided parking rate adjustment does not appear to be accurate in its depiction of reliance upon the Fourth Edition ITE to result in 11 spaces for this project and appears to be under-

parked. The Fourth Edition ITE calculations table indicates that for a 92,700 square-foot mini-warehouse facility, the average weekday demand is 13 spaces, and the 85th percentile demand would be 16 spaces.

Please submit a revised parking rate adjustment analysis and revised site plan, if applicable based upon the analysis, to ensure adequate parking is provided. For additional guidance, please see Exhibit 1 to this staff report which is provided as an example of a parking rate adjustment analysis for a similar usage that was recently accepted and recommended for approval by planning staff.

MEP RESPONSE: Acknowledged. Please see revised parking study and plans.

Item #3:

Elective Resubmittal Fee

An additional fee is required for the review of the elective resubmittal of documents for this development application. Please remit the \$2,282.00 elective resubmittal review fee with the resubmittal package. MARTIN COUNTY, FLA., LDR, §10.2.D.4

MEP RESPONSE: Acknowledged. Please see attached check.

Additional Information:

Information #1:

Notice Of A Public Hearing

The notice of a public hearing regarding development applications shall be mailed at least 14 calendar days (seven calendar days if the application is being expedited pursuant to section 10.12) prior to the public hearing by the applicant to all owners of real property located within a distance of 500 feet of the boundaries of the affected property. For development parcels which lie outside of or border the primary urban service district, the notification distance shall be increased to 1000 feet. In addition, notice shall be mailed to all homeowner associations, condominium associations and the owners of each condominium unit within the notice area. MARTIN COUNTY, FLA., LDR, SECTION 10.6.E.1. (2016)

MEP RESPONSE: Acknowledged.

Information #2:

Notice(s) of public hearings regarding development applications shall be published at least 14 days prior to the date of the public hearing (seven calendar days if the application is being expedited pursuant to section 10.12) in the legal advertisement section of a newspaper of general circulation in Martin County. The applicant shall reimburse the County for the cost(s) of the newspaper ad(s) as a post approval requirement for the application. MARTIN COUNTY, FLA., LDR SECTION 10.6.D. (2016)

MEP RESPONSE: Acknowledged.

Information #3:

Once everyone has signed off with a comply, the project will be scheduled for the next LPA meeting dependent upon the County's scheduling policy. Following the LPA meeting, the project will be scheduled for the next BCC meeting dependent upon the County's scheduling policy. MARTIN COUNTY, FLA., LDR SECTIONS 10.4. AND 10.5. (2016)

MEP RESPONSE: Acknowledged.

Information #4:
Required Permits

The applicant has elected 'Option 2' regarding Agency permit submittal for a consistency review after project approval. Prior to scheduling the mandatory pre-construction meeting for construction commencement authorization, all applicable local, state, and federal approved permits are to be submitted for review by the County Administrator with remittance of a \$600.00 review fee. If an application is made to any permitting agency for a modification to a permit that was required to be issued prior to final site plan approval, the application for the permit modification must be submitted concurrently to Martin County. MARTIN COUNTY, FLA., LDR, SECTION 10.9.A. (2012)

MEP RESPONSE: Acknowledged.

***H. Determination of compliance with the urban design and community redevelopment requirements
– Community Development Department***

Commercial Design

Findings of Compliance:

The proposed development complies with the requirements of Article 4, DIVISION 20 - Commercial Design - of the Martin County, Florida, Land Development Regulations.

Community Redevelopment Area

The proposed project is not located within a Community Redevelopment Area. Therefore, the Community Redevelopment Area reviewer was not required to review this application. MARTIN COUNTY, FLA., LDR ARTICLE 3, DIVISION 6 (2016)

***I. Determination of compliance with the property management requirements – Engineering
Department***

No dedication of additional right of way is required or proposed by the Applicant pursuant to the Roadway Classifications set forth in Section 4.843.B, Land Development Regulations, Martin County, Fla. (2001) which includes Table 4.19.1 that lists the minimum right-of-way requirements. Therefore, the Applicant is not required to submit due diligence materials for review by Real Property Management.

***J. Determination of compliance with environmental and landscaping requirements - Growth
Management Department***

Environmental

Findings of Compliance:

The Growth Management Department Environmental Division staff has reviewed the application and finds it in compliance with the applicable land development regulations. The existing upland preserve area will remain in its original configuration and protected during construction. In accordance with the approved PAMP, all exotic vegetation shall be removed prior to issuance of a Certificate of Occupancy on the first building permit.

Landscaping

Findings of Compliance:

The Growth Management Department staff has reviewed the application and finds it in compliance with the applicable Land Development Regulations regarding landscaping. The applicant has proposed construction of a residential storage facility. The applicant has submitted landscape plans that provide 48,029 s.f. of landscape area which equates to 32.5% of the 147,917 s.f. development area to document compliance with Section 4.663.A.1., Land Development Regulations, Martin County, Fla. (2013). Pursuant to this regulation a minimum of 20% of the total development area shall be landscaped.

Section 4.663.A.3.b. Land Development Regulations, Martin County, Fla. (2013) requires that all nonresidential development provide at least one tree per 2,500 sq. ft. of site area; a total of 59 trees for this project. To demonstrate compliance the applicant has proposed the planting of 95 trees, 72 palms, and preservation of 1 existing pine and 11 palms (3:1 tree credits) for this 147,917 sq. ft. site.

Landscaped bufferyards are required between differing land uses and along certain transportation corridors. Martin County, Fla Section 4.663.B.1.a, (2013). Surrounding land use on the west and part of the east is residential which requires a Type 3 Buffer. A Type 1 buffer is required on the east adjacent to a commercial project. The buffers have been provided and planted with 90 trees and 2,357 shrubs.

Section 4.666.E. Land Development Regulations, Martin County, Fla. (2013) requires that development activity preserve at least ten percent of the total number of protected trees on the site unless it can be shown that the property would be precluded of reasonable use if the trees are not removed. Due to substantial grade changes necessary, to meet this requirement the applicant has submitted Landscape and Construction Plans to provide for removal of 14 existing trees and preservation of 1 large slash pine and 16 sabal palm within the proposed perimeter landscape areas. To demonstrate compliance with Section 4.666.D., Land Development Regulations, Martin County, Fla. (2013) the applicant has proposed installation of #50 2-inch caliper native trees equal to mitigate for the necessary tree removal.

Section 4.663.A.4.b.1, 2, , and 3., Land Development Regulations, Martin County, Fla. (2013) requires one 500 s.f. landscape area with 3 – 2” caliper or 2 – 3” caliper trees for each 5000 s.f. of interior vehicular use area. This project has 40,685 s.f. of paving and the applicant is proposing the installation of 12 – 2”, 6 – 3” caliper native trees and 15 sabals within the vehicular use area of the site.

Alterations cannot be made to the plans after final site plan approval. Any alteration may require an application to amend the affected approved plans.

The applicant is cautioned to consider the placement of utilities and any underground or above ground site improvement that could cause a conflict with the landscaping and possibly cause a change or amendment.

As-built landscape plans submitted prior to the release of a certificate of occupancy will be checked against the approved drawings. Inconsistencies may block the issuance of the certificate of occupancy and cause the applicant to begin the application process for a change or an amendment to the development order.

K. Determination of compliance with transportation requirements - Engineering Department

Findings of Compliance:

The Traffic Division of the Engineering Department finds this application in compliance.

Compliance with Adequate Public Facilities Ordinance:

Staff has reviewed the Traffic Statement prepared by MacKenzie Engineering & Planning, dated December 2017. MacKenzie Engineering & Planning stated that the site's maximum impact was assumed to be 8 directional trips during the PM peak hour. Staff finds that Jensen Beach Boulevard is the recipient of a majority of the generated trips. The generalized service capacity of Jensen Beach Boulevard is 2000. The project impact is 0.4% of the maximum volume of that roadway. Jensen Beach Boulevard is currently operating at a level of service C; it is anticipated to operate at level of service C at buildout (year 2020).

This application satisfies the Adequate Public Facilities Standard; it has a De Minimis impact (an impact that would not affect more than one percent of the maximum volume at the adopted level of service of the accepted road facility) (Article 5, Division 1, Section 5.3).

L. Determination of compliance with county surveyor - Engineering Department

The applicant has provided a certified boundary and topographic survey for the proposed development, pursuant to Section 10.1.F., LDR, Martin County, Fla. (2016). Therefore, the Engineering Department was not required to review this application for consistency with the Martin County Codes for survey requirements contained in Article 4, LDR, Martin County, Fla.

M. Determination of compliance with engineering, storm water and flood management requirements - Engineering Department

Unresolved Issues:

Item #1:

CONSTRUCTION PLANS

1. As previously requested, label the 6' FPL Easement at the north property line on all sheets of the Construction Plans.

MEP RESPONSE: Acknowledged. Please see revised construction plans on all sheets.

2. As previously requested, clearly identify and label all proposed curbs and gutter types on the Revised Final Site Plan (SP-1) and the Paving, Grading and Drainage Plan (C-01)

MEP RESPONSE: Acknowledged. Please see revised Site and PGD Plans, sheets SP-1 and C-01.

3. As previously requested, remove proposed curbing from the entrance radii and transition the site curb to form the accessible ramp curbing at the existing sidewalk. Match existing Jensen Beach Blvd edge of pavement and slope the entrance driveway, within the right of way, for storm drainage flow to the existing swale. Revise line work on the Revised Final Site Plan and sheet C-01 accordingly.

MEP RESPONSE: Acknowledged. The Radii matches edge of pavement with the addition of bike lane by FDOT.

4. As previously requested, label the accessible ramp locations and FDOT index numbers to all ADA ramps on the Paving, Grading and Drainage Plan, sheet C-01.

MEP RESPONSE: Acknowledged. Please see revised PGD Plan, sheet C-01.

5. As previously requested, provide a catch basin temporary erosion and sediment control detail (FDOT Index 102).

MEP RESPONSE: Acknowledged. Please see revised PGD Plan, sheet C-02.

6. Provide a gate access within the perimeter wall to provide direct access to the proposed control structure for maintenance; revise accordingly [MARTIN COUNTY FLA LDR SECTION 4.348.H.].

MEP RESPONSE: Acknowledged. Please see revised construction plans on all sheets.

7. As previously requested, provide details showing how the concrete flume (located at the south end of the west stormwater retention area) interacts with the proposed perimeter wall.

MEP RESPONSE: Acknowledged. This flume will be installed 6 inches below the proposed Opaque PVC fence, please refer to detail on sheet C-02.

Item #2:

STORMWATER MANAGEMENT REPORT

1. Provide the necessary documentation supporting the wet season water table elevation. The wet season water table shall be the highest water table described in either the "Detailed Soil Map Units" section or Table 17 "Water Features" of the USDA Soil Survey of Martin County Area, Florida. A different water table elevation may be used if competent evidence prepared by a professional engineer licensed in the State of Florida demonstrates, to the satisfaction of the County Engineer, that the water table is different from that shown in the soil survey. [MARTIN COUNTY FLA LDR SECTION 4.384.A.3.a.4 (2001)]

TEG RESPONSE: Acknowledged. Please see additional documentation. These include surrounding SFWMD permit documentation demonstrating the control elevation around this site is at or below 8.50NAVD.

2. Demonstrate how the minimum perimeter berm elevation is met along the southern property line within the preserve area [MARTIN COUNTY FLA LDR SECTION 4.384].

TEG RESPONSE: Acknowledged. This is now specified on sheet C-03 of the plans.

3. Revise the Final Site Plan finished floor elevations to be consistent with the Construction Plans. [MARTIN COUNTY FLA LDR SECTION 4.385.B.15].

TEG RESPONSE: Acknowledged. Please see revised site plan, sheet SP-1.

4. Martin County prohibits the use of infiltration when designing a site for stormwater attenuation and flood protection purposes. Therefore, the proposed exfiltration trench must only be used for the purposes of water quality design. The storage volume provided within the trench must be excluded ICPR modeling, and the 100-year 3-day zero discharge calculation. Revise the stormwater calculations accordingly.

TEG RESPONSE: Acknowledged. We removed the exfiltration trench from the construction drawings as full water quality is accommodated by the dry-detention area.

5. Revise the water quality calculation to provide for 3-inches of rainfall over the percent of impervious project area (4.386.I) (total impervious area less lakes, preserves, and wetlands; roof areas (roof areas are not included in the Stormwater Report) are included); $[\% \text{ imperv} = (\text{Roof} + \text{Pavt}) / (\text{Total} - \text{Lakes} - \text{Preserves} - \text{Wetlands})]$; a 50% treatment credit is not applicable to Martin County water quality requirements. [MARTIN COUNTY FLA LDR SECTION 4.385.F.4 (2015)].

TEG RESPONSE: Please see Stormwater Report revised November 2018. Area of buildings and impervious surface totals 2.29ac. This area is utilized in the calculation. Please note, the 50% credit has been removed and replaced with 125% of the volume due to dry detention being utilized.

400