SOIL BORING, SAMPLING AND TESTING METHODS

(abbreviated version for project specific methods and soil conditions)

GENERAL

Andersen Andre Consulting Engineers, Inc. (AACE) borings describe subsurface conditions only at the locations drilled and at the time drilled. They provide no information about subsurface conditions below the bottom of the boreholes. At locations not explored, surface conditions that differ from those observed in the borings may exist and should be anticipated.

The information reported on our boring logs is based on our drillers' logs and on visual examination in our laboratory of disturbed soil samples recovered from the borings. The distinction shown on the logs between soil types is approximate only. The actual transition from one soil to another may be gradual and indistinct.

The groundwater depth shown on our boring logs is the water level the driller observed in the borehole when it was drilled. These water levels may have been influenced by the drilling procedures, especially in borings made by rotary drilling with bentonitic drilling mud. An accurate determination of groundwater level requires long-term observation of suitable monitoring wells. Fluctuations in groundwater levels throughout the year should be anticipated.

The absence of a groundwater level on certain logs indicates that no groundwater data is available. It does not mean that groundwater will not be encountered at that boring location at some other point in time.

HAND AUGER BORINGS

Hand auger borings are used, if soil conditions are favorable, when the soil strata are to be determined within a shallow (approximately 5-foot [1.5m]) depth or when access is not available to power drilling equipment. A 3-inch (75mm) diameter hand bucket auger with a cutting head is simultaneously turned and pressed into the ground. The bucket auger is retrieved at approximately 6-inch (0.15m) interval and its contents emptied for inspection. On occasion post-hole diggers are used, especially in the upper 3 feet (1m) or so. Penetrometer probings can be used in the upper 5 feet (1.5m) to determine the relative density of the soils. The soil sample obtained is described and representative samples put in bags or jars and transported to the AACE soils laboratory for classification and testing, if necessary.

STANDARD PENETRATION TEST

The Standard Penetration Test (SPT) is a widely accepted method of in situ testing of foundation soils (ASTM D-1586). A 2-foot (0.6m) long, 2-inch (50mm) O.D. split-barrell sampler attached to the end of a string of drilling rods is driven 24 inches (0.60m) into the ground by successive blows of a 140-pound (63.5 Kg) hammer freely dropping 30 inches (0.76m). The number of blows needed for each 6 inches (0.15m) increments penetration is recorded. The sum of the blows required for penetration of the middle two 6-inch (0.15m) increments of penetration constitutes the test result of N-value. After the test, the sampler is extracted from the ground and opened to allow visual description of the retained soil sample. The N-value has been empirically correlated with various soil properties allowing a conservative estimate of the behavior of soils under load. The following tables relate N-values to a qualitative description of soil density for cohesionless soils:

Cohesionless Soils:	N-Value	Description
	0 to 4	Very loose
	4 to 10	Loose
	10 to 30	Medium dense
	30 to 50	Dense
	Above 50	Very dense

Cohesive Soils:	N-Value	Description	Qu
	0 to 2	Very soft	Below 0.25 tsf (25 kPa)
	2 to 4	Soft	0.25 to 0.50 tsf (25 to 50 kPa)
	4 to 8	Medium stiff	0.50 to 1.0 tsf (50 to 100 kPa)
	8 to 15	Stiff	1.0 to 2.0 tsf (100 to 200 kPa)
	15 to 30	Very stiff	2.0 to 4.0 tsf (200 to 400 kPa)
	Above 30	Hard	Above 4.0 tsf (400 kPa)

The tests are usually performed at 5 foot (1.5m) intervals. However, more frequent or continuous testing is done by AACE through depths where a more accurate definition of the soils is required. The test holes are advanced to the test elevations by rotary drilling with a cutting bit, using circulating fluid to remove the cuttings and hold the fine grains in suspension. The circulating fluid, which is bentonitic drilling mud, is also used to keep the hole open below the water table by maintaining an excess hydrostatic pressure inside the hole. In some soil deposits, particularly highly pervious ones, flush-coupled casing must be driven to just above the testing depth to keep the hole open and/or prevent the loss of circulating fluid. After completion of a test borings, the hole is kept open until a steady state groundwater level is recorded. The hole is then sealed by backfilling, either with accumulated cuttings or lean cement.

Representative split-spoon samples from each sampling interval and from different strata are brought to our laboratory in air-tight jars for classification and testing, if necessary. Afterwards, the samples are discarded unless prior arrangement have been made.

SFWMD EXFILTRATION TESTS (USUAL CONDITION TEST)

In order to estimate the hydraulic conductivity of the upper soils, constant head or falling head exfiltration tests can be performed. These tests are performed in accordance with methods described in the South Florida Water Management District (SFWMD) Permit Information Manual, Volume IV. In brief, for the "Usual Condition Test", a 6 to 9 inch diameter hole is augered to depths of about 5 to 7 feet; the bottom one foot is filled with 57-stone; and a 6-foot long slotted PVC pipe is lowered into the hole. The distance from the groundwater table and to the ground surface is recordedand the hole is then saturated for 10 minutes with the water level maintained at the ground surface.

If a constant head test is performed, the rate of pumping will be recorded at fixed intervals of 1 minute for a total of 10 minutes, following the saturation period.

LABORATORY TEST METHODS

Soil samples returned to the AACE soils laboratory are visually observed by a geotechnical engineer or a trained technician to obtain more accurate description of the soil strata. Laboratory testing is performed on selected samples as deemed necessary to aid in soil classification and to help define engineering properties of the soils. The test results are presented on the soil boring logs at the depths at which the respective sample was recovered, except that grain size distributions or selected other test results may be presented on separate tables, figures or plates as discussed in this report. The soil descriptions shown on the logs are based upon visual-manual procedures in accordance with local practice. Soil classification is performed in general accordance with the United Soil Classification System (ASTM D-2487) and is also based on visual-manual procedures.

THE PROJECT SOIL DESCRIPTION PROCEDURE FOR SOUTHEAST FLORIDA

For use with the ASTM D-2487 Unified Soil Classification System

CLASSIFICATION OF SOILS FOR ENGINEERING PURPOSES

BOULDERS (>12" [300 MM]) and COBBLES (3" [75 MM] TO 12" [300 MM]):

GRAVEL: Coarse Gravel: 3/4" (19 mm) to 3" (75 mm)
Fine Gravel: No. 4 (4.75 mm) Sieve to 3/4" (19 mm)

Descriptive adjectives:
0 - 5% - no mention of gravel in description
5 - 15% - trace
15 - 29% - some
30 - 49% - gravelly (shell, limerock, cemented sands)

SANDS:

COARSE SAND: No. 10 (2 mm) Sieve to No. 4 (4.75 mm) Sieve
MEDIUM SAND: No. 40 (425 μm) Sieve to No. 10 (2 mm) Sieve
FINE SAND: No. 200 (75 μm) Sieve to No. 40 (425 μm) Sieve

Descriptive adjectives:
0 - 5%- no mention of sand in description
5 - 15% - trace
15 - 29% - some
30 - 49% - sandy

SILT/CLAY: < #200 (75/μm) Sieve

SILTY OR SILT: PI < 4
SILTY CLAYEY OR SILTY CLAY: 4 $\leq$ PI $\leq$ 7
CLAYEY OR CLAY: PI > 7

Descriptive adjectives:
< 5% - clean (no mention of silt or clay in description)
5 - 15% - slightly
16 - 35% - clayey, silty, or silty clayey
36 - 49% - very

ORGANIC SOILS:

Organic Content	Descriptive Adjectives	Classification
0 - 2.5%	Usually no mention of org.	See Above
2.6 - 5%	slightly organic	add "with organic fines" to group name
5 - 30%	organic	SM with organic fines
		Organic Silt (OL)
		Organic Clay (OL)
		Organic Silt (OH)
		Organic Clay (OH)

NOTES:

TB-#	STANDARD PENETRATION TEST (SPT) BORING [ASTM D1586]
AB-#	AUGER BORING (ASTM D1452)
EX-#	SFWMD EXFILTRATION TEST
N	SPT RESISTANCE IN BLOWS PER FOOT
X,X' ▽	GROUNDWATER TABLE (GWT) MEASURED ON THE DATE DRILLED
N.E.	NOT ENCOUNTERED
EOB	END OF BORING
BLS	BELOW LAND SURFACE
SP, SP-SM, ETC:	UNIFIED SOIL CLASSIFICATION SYSTEM (USCS) USCS GROUPS DETERMINED BY VISUAL CLASSIFICATION, EXCEPT FOR NOTED LABORATORY TEST RESULTS.
MC	NATURAL MOISTURE CONTENT IN PERCENT [ASTM D2216]
OC	ORGANIC CONTENT IN PERCENT [ASTM D2974]
-200	PERCENT PASSING NO. 200 SIEVE SIZE (PERCENT FINES) [ASTM D1140]

SPT BORING DATA
DRILL CREW: CL/AACE
DRILL RIG: CME-45
DRILL METHOD: ROTARY-WASH W. BENTONITE DRILLING SLURRY
SPT DATA:
SPOON I.D. = 1.375"
SPOON O.D. = 2.0"
HAMMER DROP = 30"
HAMMER WEIGHT = 140 lbs.
HAMMER TYPE = MANUAL

SFWMD EXFILTRATION TESTS

Q	STABILIZED FLOW RATE
d	DIAMETER OF TEST HOLE
H2	HYDROSTATIC COLUMN
Ds	SATURATED HOLE DEPTH (BY GWT)
	NOTE: IF 0, GWT NOT ENCOUNTERED

LEGEND:

TOPSOIL

FINE SAND (SP)

SLIGHTLY SILTY FINE SAND (SP-SM) W. HARDPAN FRAGMENTS [HARDPAN-TYPE]

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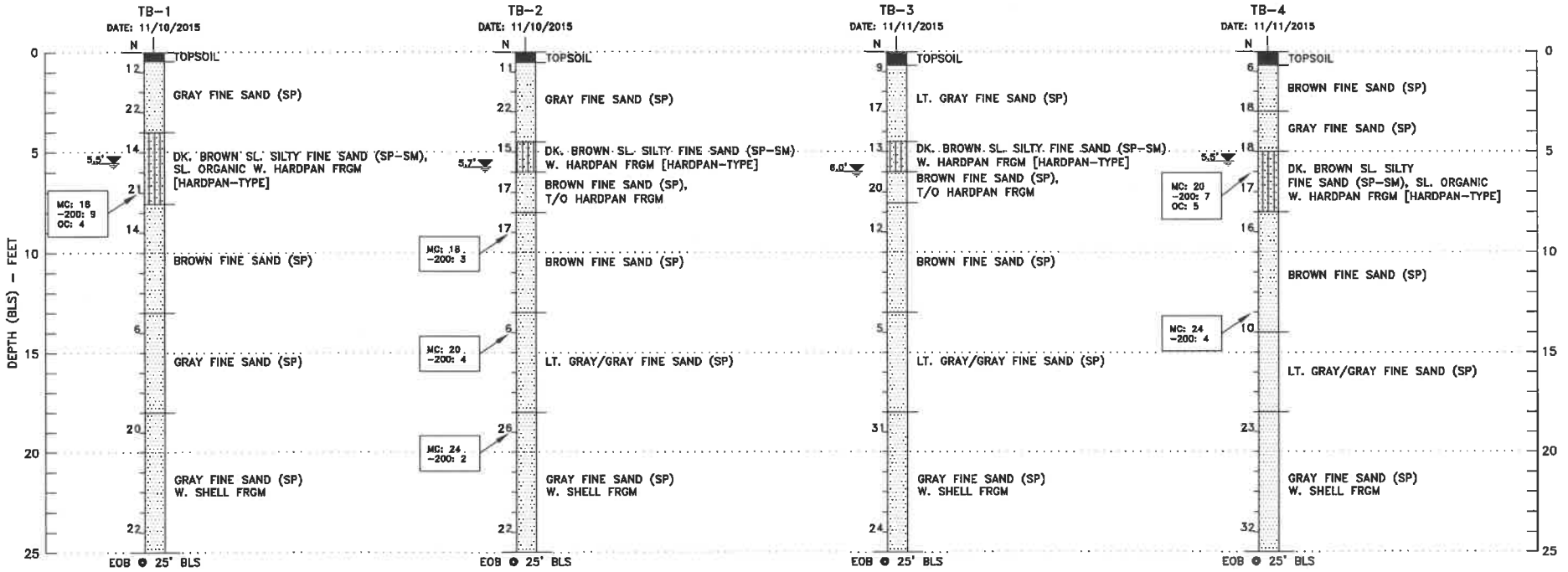
GENERAL NOTES

GEOTECHNICAL ENGINEERING EVALUATION
MARINER VILLAGE SQUARE
MARTIN OUNTY, FLORIDA

Drawn by: PGA	Date: November 2015
Checked by: DPA	Date: November 2015
AACE File No: 15-185	Sheet No. 1



LEGEND:

- TOPSOIL
- FINE SAND (SP)
- SLIGHTLY SILTY FINE SAND (SP-SM) W. HARDPAN FRAGMENTS [HARDPAN-TYPE]

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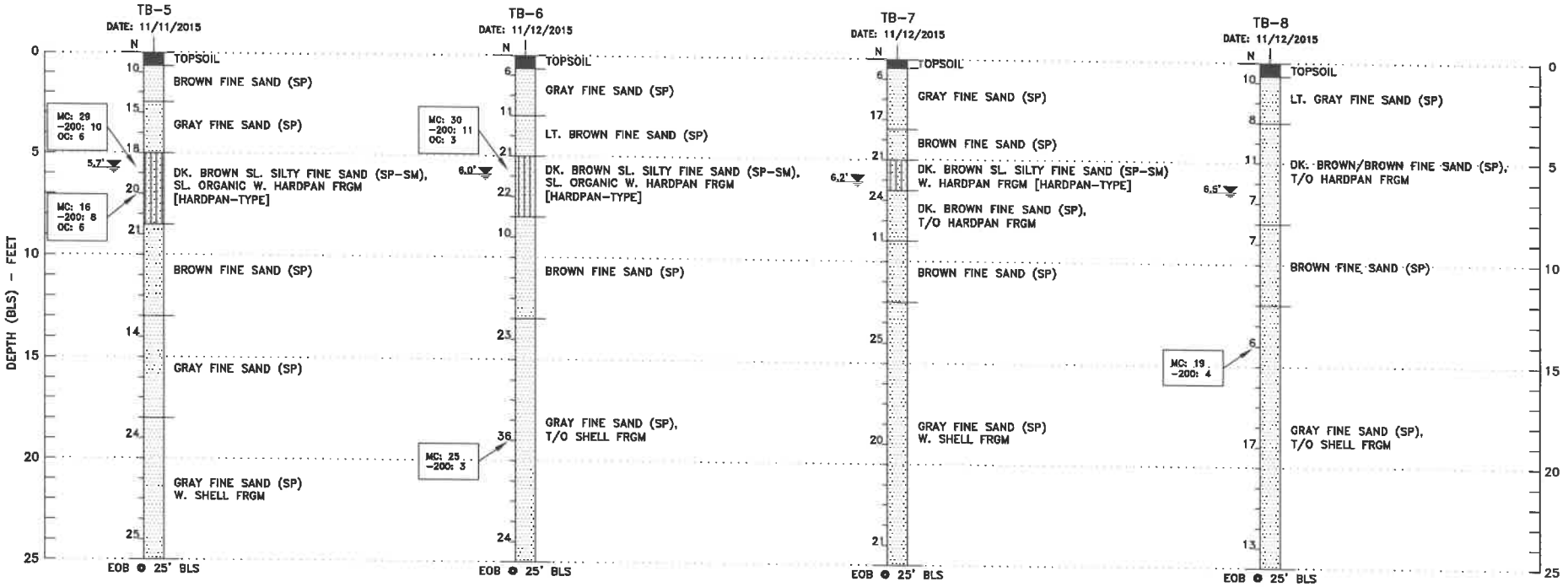
SOIL BORING PROFILES

GEOTECHNICAL ENGINEERING EVALUATION
MARINER VILLAGE SQUARE
MARTIN OUNTY, FLORIDA

Drawn by: PGA	Date: November 2015
Checked by: DPA	Date: November 2015
AACE File No: 15-185	**Sheet No. 2**



LEGEND:

- TOPSOIL
- FINE SAND (SP)
- SLIGHTLY SILTY FINE SAND (SP-SM) W. HARDPAN FRAGMENTS [HARDPAN-TYPE]

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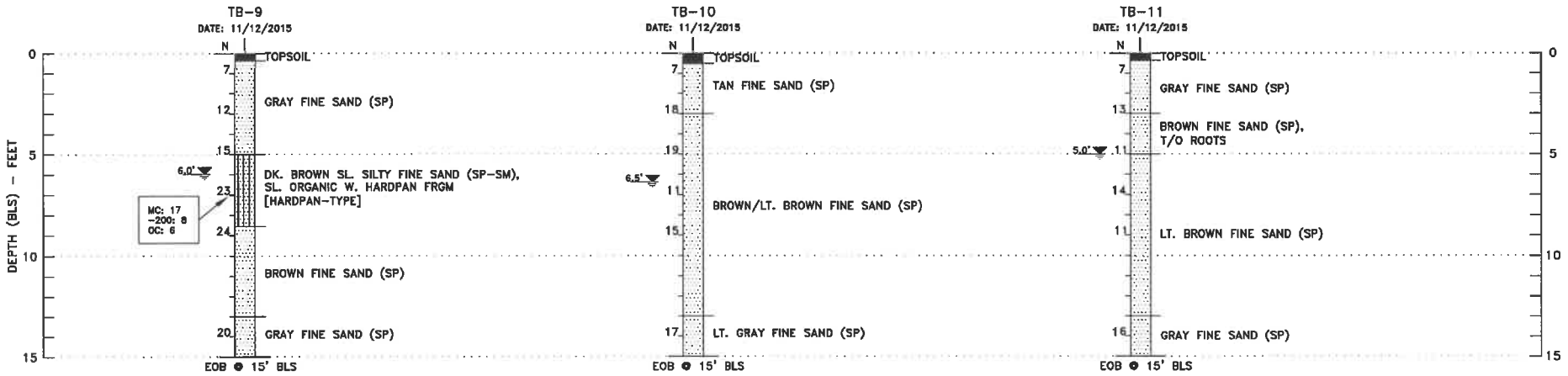
SOIL BORING PROFILES

GEOTECHNICAL ENGINEERING EVALUATION
MARINER VILLAGE SQUARE
MARTIN OUNTY, FLORIDA

Drawn by: PGA	Date: November 2015
Checked by: DPA	Date: November 2015
AACE File No: 15-185	**Sheet No. 3**



LEGEND:

- TOPSOIL
- FINE SAND (SP)
- SLIGHTLY SILTY FINE SAND (SP-SM) W. HARDPAN FRAGMENTS [HARDPAN-TYPE]

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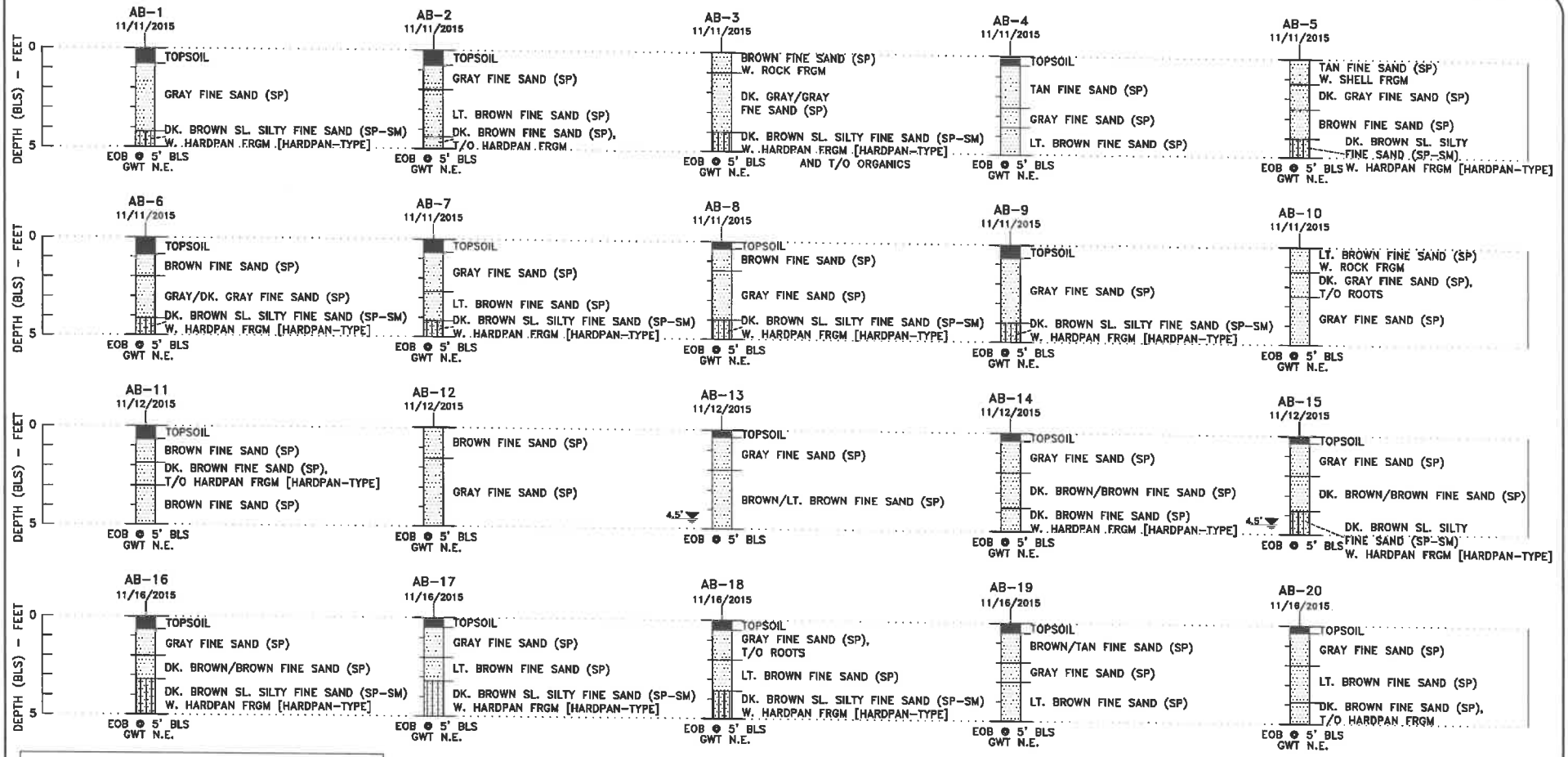
SOIL BORING PROFILES

GEOTECHNICAL ENGINEERING EVALUATION
MARINER VILLAGE SQUARE
MARTIN OUNTY, FLORIDA

Drawn by: PGA	Date: November 2015
Checked by: DPA	Date: November 2015
AACE File No: 15-185	**Sheet No. 4**



LEGEND:

- TOPSOIL
- FINE SAND (SP)
- SLIGHTLY SILTY FINE SAND (SP-SM) W. HARDPAN FRAGMENTS [HARDPAN-TYPE]

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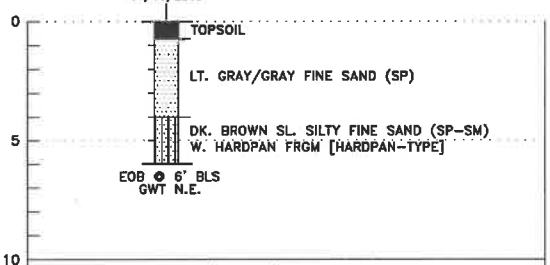
SOIL BORING PROFILES

GEOTECHNICAL ENGINEERING EVALUATION
MARINER VILLAGE SQUARE
MARTIN OUNTY, FLORIDA

Drawn by: PGA	Date: November 2015
Checked by: DPA	Date: November 2015
AACE File No: 15-185	**Sheet No. 5**

EX-1
11/11/2015



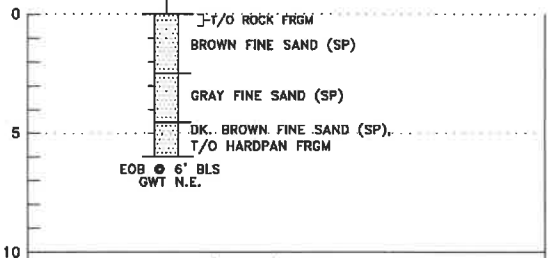
TEST SUMMARY (EX-1):

Q = 7.1×10^{-3} CFS
d = 0.5 FT
H_2 = 6+ FT
Ds = 0 FT

K = 2.4×10^{-4} CFS/SQ.FT. - FT. HD.

This value is an ultimate value and an appropriate factor of safety should be used in the design of drainage improvements.

EX-2
11/11/2015



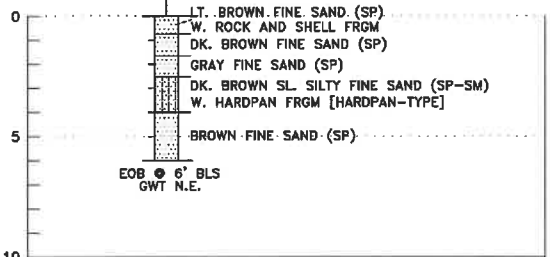
TEST SUMMARY (EX-2):

Q = 8.9×10^{-3} CFS
d = 0.5 FT
H_2 = 6+ FT
Ds = 0 FT

K = 3.0×10^{-4} CFS/SQ.FT. - FT. HD.

This value is an ultimate value and an appropriate factor of safety should be used in the design of drainage improvements.

EX-3
11/12/2015



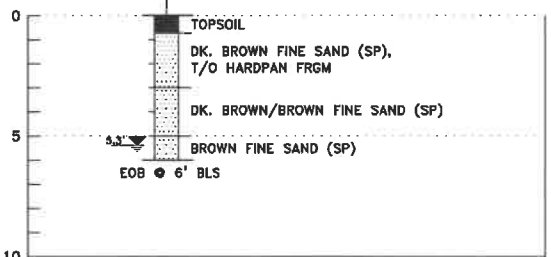
TEST SUMMARY (EX-3):

Q = 5.8×10^{-3} CFS
d = 0.5 FT
H_2 = 6+ FT
Ds = 0 FT

K = 2.0×10^{-4} CFS/SQ.FT. - FT. HD.

This value is an ultimate value and an appropriate factor of safety should be used in the design of drainage improvements.

EX-4
11/12/2015



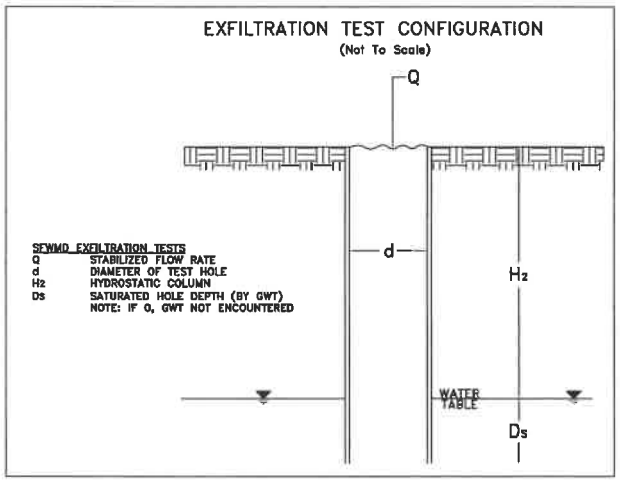
TEST SUMMARY (EX-4):

Q = 4.2×10^{-3} CFS
d = 0.5 FT
H_2 = 5.3 FT
Ds = 0.7 FT

K = 1.4×10^{-4} CFS/SQ.FT. - FT. HD.

This value is an ultimate value and an appropriate factor of safety should be used in the design of drainage improvements.

EXFILTRATION TEST CONFIGURATION
(Not To Scale)



LEGEND:

- TOPSOIL
- FINE SAND (SP)
- SLIGHTLY SILTY FINE SAND (SP-SM) W. HARDPAN FRAGMENTS [HARDPAN-TYPE]

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EXFILTRATION TEST RESULTS

GEOTECHNICAL ENGINEERING EVALUATION
MARINER VILLAGE SQUARE
MARTIN OUNTY, FLORIDA

Drawn by: PGA	Date: November 2015
Checked by: DPA	Date: November 2015
AACE File No: 15-185	Sheet No. 6

APPENDIX I

USDA Soil Survey Information

Soil Map—Martin County, Florida
(Mariner Square Village)

Map Scale: 1:5,130 if printed on A landscape (11" x 8.5") sheet.

N

Meters
0 50 100 200 300

Feet
0 200 400 800 1200

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 17N WGS84

MAP LEGEND

Area of Interest (AOI)
- Area of Interest (AOI)

Soils
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features
- Streams and Canals

Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background
- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Martin County, Florida
Survey Area Data: Version 13, Sep 21, 2015

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Feb 14, 2015—May 8, 2015

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Martin County, Florida (FL085)			
Map Unit Symbol	**Map Unit Name**	**Acres in AOI**	**Percent of AOI**
4	Waveland and Immokalee fine sands	87.7	82.9%
13	Placid and Basinger fine sands, depressional	15.2	14.3%
99	Water	3.0	2.8%
Totals for Area of Interest		**105.8**	**100.0%**

Martin County, Florida

4—Waveland and Immokalee fine sands

Map Unit Setting

National map unit symbol: 1jq7n
Mean annual precipitation: 56 to 64 inches
Mean annual air temperature: 72 to 79 degrees F
Frost-free period: 350 to 365 days
Farmland classification: Farmland of unique importance

Map Unit Composition

Immokalee and similar soils: 40 percent
Waveland and similar soils: 40 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Waveland

Setting

Landform: Flatwoods on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Sandy marine deposits

Typical profile

A - 0 to 4 inches: fine sand
Eg - 4 to 43 inches: fine sand
Bh1 - 43 to 47 inches: fine sand
Bh2 - 47 to 77 inches: loamy fine sand
Cg1 - 77 to 91 inches: fine sand
Cg2 - 91 to 99 inches: fine sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: 30 to 50 inches to ortstein
Natural drainage class: Poorly drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: About 6 to 18 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Very low (about 1.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4w
Hydrologic Soil Group: A/D
Other vegetative classification: South Florida Flatwoods (R156BY003FL), Sandy soils on flats of mesic or hydric lowlands (G156BC141FL)

Description of Immokalee

Setting

Landform: Flatwoods on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Convex
Across-slope shape: Linear
Parent material: Sandy marine deposits

Typical profile

A - 0 to 6 inches: fine sand
E - 6 to 35 inches: fine sand
Bh - 35 to 54 inches: fine sand
BC - 54 to 80 inches: fine sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Poorly drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 1.98 in/hr)
Depth to water table: About 6 to 18 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Low (about 5.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4w
Hydrologic Soil Group: B/D
Other vegetative classification: South Florida Flatwoods (R156BY003FL), Sandy soils on flats of mesic or hydric lowlands (G156BC141FL)

Minor Components

Basinger

Percent of map unit: 4 percent
Landform: Drainageways on marine terraces
Landform position (three-dimensional): Dip
Down-slope shape: Linear
Across-slope shape: Concave
Other vegetative classification: Slough (R156BY011FL), Sandy soils on flats of mesic or hydric lowlands (G156BC141FL)

Lawnwood

Percent of map unit: 4 percent
Landform: Marine terraces on flatwoods
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Other vegetative classification: South Florida Flatwoods (R156BY003FL), Sandy soils on flats of mesic or hydric lowlands (G156BC141FL)

Placid

Percent of map unit: 3 percent
Landform: Depressions on marine terraces
Landform position (three-dimensional): Dip
Down-slope shape: Concave
Across-slope shape: Concave
Other vegetative classification: Freshwater Marshes and Ponds (R156BY010FL), Sandy soils on stream terraces, flood plains, or in depressions (G156BC145FL)

Jonathan

Percent of map unit: 3 percent
Landform: Rises on marine terraces
Landform position (three-dimensional): Interfluve
Down-slope shape: Convex
Across-slope shape: Linear
Other vegetative classification: South Florida Flatwoods (R156BY003FL), Sandy soils on rises, knolls, and ridges of mesic uplands (G156BC121FL)

Nettles

Percent of map unit: 3 percent
Landform: Flatwoods on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Other vegetative classification: South Florida Flatwoods (R156BY003FL), Sandy soils on flats of mesic or hydric lowlands (G156BC141FL)

Salerno

Percent of map unit: 3 percent
Landform: Flatwoods on marine terraces
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear

Other vegetative classification: South Florida Flatwoods (R156BY003FL), Sandy soils on flats of mesic or hydric lowlands (G156BC141FL)

Data Source Information

Soil Survey Area: Martin County, Florida
Survey Area Data: Version 13, Sep 21, 2015

Martin County, Florida

13—Placid and Basinger fine sands, depressional

Map Unit Setting

National map unit symbol: 1jq7x
Mean annual precipitation: 56 to 64 inches
Mean annual air temperature: 72 to 79 degrees F
Frost-free period: 350 to 365 days
Farmland classification: Farmland of unique importance

Map Unit Composition

Placid and similar soils: 45 percent
Basinger and similar soils: 40 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Placid

Setting

Landform: Depressions on marine terraces
Landform position (three-dimensional): Dip
Down-slope shape: Concave
Across-slope shape: Concave
Parent material: Sandy marine deposits

Typical profile

A - 0 to 17 inches: fine sand
Cg - 17 to 80 inches: fine sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: About 0 inches
Frequency of flooding: None
Frequency of ponding: Frequent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Moderate (about 6.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7w
Hydrologic Soil Group: A/D

Other vegetative classification: Freshwater Marshes and Ponds (R156BY010FL), Sandy soils on stream terraces, flood plains, or in depressions (G156BC145FL)

Description of Basinger

Setting

Landform: Depressions on marine terraces
Landform position (three-dimensional): Dip
Down-slope shape: Concave
Across-slope shape: Concave
Parent material: Sandy marine deposits

Typical profile

A - 0 to 4 inches: fine sand
Eg - 4 to 22 inches: fine sand
Bh/Eg - 22 to 42 inches: fine sand
Cg - 42 to 80 inches: fine sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: About 0 inches
Frequency of flooding: None
Frequency of ponding: Frequent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Low (about 5.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7w
Hydrologic Soil Group: A/D
Other vegetative classification: Freshwater Marshes and Ponds (R156BY010FL), Sandy soils on stream terraces, flood plains, or in depressions (G156BC145FL)

Minor Components

Lawnwood

Percent of map unit: 8 percent
Landform: Marine terraces on flatwoods
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Other vegetative classification: South Florida Flatwoods (R156BY003FL), Sandy soils on flats of mesic or hydric lowlands (G156BC141FL)

Sanibel

Percent of map unit: 7 percent
Landform: Depressions on marine terraces
Landform position (three-dimensional): Dip
Down-slope shape: Concave
Across-slope shape: Concave
Other vegetative classification: Freshwater Marshes and Ponds (R156BY010FL), Organic soils in depressions and on flood plains (G156BC645FL)

Data Source Information

Soil Survey Area: Martin County, Florida
Survey Area Data: Version 13, Sep 21, 2015

APPENDIX II

Laboratory Testing Results

ANDERSEN ANDRE CONSULTING ENGINEERS, INC.

AACE

Moisture Content (ASTM D2216), Percent Fines Passing US No. 200 Sieve (ASTM D1140)

Job No: **15-185**
Project: **Mariner Square Village**
Location: **Martin County, FL**
Station: **NA**
Date: **11/16/15**
Technician **RL**

Sample ID	Pan #	Tare weight [grams]	Wet Weight Before Wash	Dry Weight Before wash		Water Weight [grams]	Dry Weight After wash		Moisture (%)	Fines (%)
			Soil + tare weight [grams]	Soil + tare weight [grams]	Soil weight [grams]		Soil + tare weight [grams]	Soil weight [grams]		
TB-1 #4	T15	85.6	206.5	188.2	102.6	18.3	178.6	93.0	**18**	**9**
TB-4 #3	P36	86.7	207.6	187.8	101.1	19.8	180.5	93.8	**20**	**7**
TB-5 #3	P37	88.2	205.4	178.9	90.7	26.5	170.1	81.9	**29**	**10**
TB-5 #5	P43	87.2	201.1	185.7	98.5	15.4	178.0	90.8	**16**	**8**
TB-6 #3	P38	86.7	201.0	174.9	88.2	26.1	165.4	78.7	**30**	**11**
TB-9 #3	P22	86.3	195.3	179.2	92.9	16.1	172.0	85.7	**17**	**8**
TB-2 #5	T6	86.2	211.2	192.2	106.0	19.0	189.1	102.9	**18**	**3**
TB-2 #6	T17	85.7	195.6	177.0	91.3	18.6	172.9	87.2	**20**	**4**
TB-2 #7	T22	86.8	212.7	188.3	101.5	24.4	186.2	99.4	**24**	**2**
TB-4 #6	P1	86.1	207.3	183.7	97.6	23.6	179.8	93.7	**24**	**4**
TB-6 #7	P27	87.2	215.9	190.2	103.0	25.7	186.9	99.7	**25**	**3**
TB-8 #5	P23	85.9	199.3	181.0	95.1	18.3	177.1	91.2	**19**	**4**

ANDERSEN ANDRE CONSULTING ENGINEERS, INC.

Organic Content Work Sheet (AASHTO T-267 / ASTM D2974)

Project Name: **Mariner Square Village**
File Number: **15-185**
Sample Location: **Varies**
Sample Description: **Refer to Log**

USCS/AASHTO: **NA**
Date Sampled: **Varies**
Date Tested: **11/16/2015**
Tested By: **SM**

Loss On Ignition (LO) Test

	Sample ID	TB-1 #4
	Sample Location	As noted on log
	Depth	As noted on log
	Tare Number	P13
	Wt. Of Tare (g) - A	22.3
b.i.	Wt. Of Tare+Soil+Orgn (g) - B	63.8
a.i.	Wt. Tare+Soil (g) - C	62.2
	% Organics: 100x(B-C)/(B-A)	4

Loss On Ignition (LO) Test

	Sample ID	TB-4 #3
	Sample Location	As noted on log
	Depth	As noted on log
	Tare Number	P95
	Wt. Of Tare (g) - A	24.1
b.i.	Wt. Of Tare+Soil+Orgn (g) - B	60.3
a.i.	Wt. Tare+Soil (g) - C	58.3
	% Organics: 100x(B-C)/(B-A)	6

Loss On Ignition (LO) Test

	Sample ID	TB-3 #3
	Sample Location	As noted on log
	Depth	As noted on log
	Tare Number	P60
	Wt. Of Tare (g) - A	27.8
b.i.	Wt. Of Tare+Soil+Orgn (g) - B	52.8
a.i.	Wt. Tare+Soil (g) - C	51.5
	% Organics: 100x(B-C)/(B-A)	5

Loss On Ignition (LO) Test

	Sample ID	TB-6 #3
	Sample Location	As noted on log
	Depth	As noted on log
	Tare Number	P42
	Wt. Of Tare (g) - A	22.5
b.i.	Wt. Of Tare+Soil+Orgn (g) - B	59.9
a.i.	Wt. Tare+Soil (g) - C	58.8
	% Organics: 100x(B-C)/(B-A)	3

Loss On Ignition (LO) Test

	Sample ID	TB-9 #3
	Sample Location	As noted on log
	Depth	As noted on log
	Tare Number	P8
	Wt. Of Tare (g) - A	27.8
b.i.	Wt. Of Tare+Soil+Orgn (g) - B	53.0
a.i.	Wt. Tare+Soil (g) - C	51.5
	% Organics: 100x(B-C)/(B-A)	6

Loss On Ignition (LO) Test

	Sample ID	
	Sample Location	
	Depth	
	Tare Number	
	Wt. Of Tare (g) - A	
b.i.	Wt. Of Tare+Soil+Orgn (g) - B	
a.i.	Wt. Tare+Soil (g) - C	
	% Organics: 100x(B-C)/(B-A)	

Loss On Ignition (LO) Test

	Sample ID	TB-5 #5
	Sample Location	As noted on log
	Depth	As noted on log
	Tare Number	P18
	Wt. Of Tare (g) - A	24.1
b.i.	Wt. Of Tare+Soil+Orgn (g) - B	56.3
a.i.	Wt. Tare+Soil (g) - C	54.3
	% Organics: 100x(B-C)/(B-A)	6

Loss On Ignition (LO) Test

	Sample ID	
	Sample Location	
	Depth	
	Tare Number	
	Wt. Of Tare (g) - A	
b.i.	Wt. Of Tare+Soil+Orgn (g) - B	
a.i.	Wt. Tare+Soil (g) - C	
	% Organics: 100x(B-C)/(B-A)	

Notes: b.i - before ignition, a.i - after ignition
report organics to 0.1%

APPENDIX III

AACE Project Limitations and Conditions

ANDERSEN ANDRE CONSULTING ENGINEERS, INC.

(revised January 24, 2007)

Project Limitations and Conditions

Andersen Andre Consulting Engineers, Inc. has prepared this report for our client for his exclusive use, in accordance with generally accepted soil and foundation engineering practices. No other warranty, expressed or implied, is made herein. Further, the report, in all cases, is subject to the following limitations and conditions:

VARIABLE/UNANTICIPATED SUBSURFACE CONDITIONS

The engineering analysis, evaluation and subsequent recommendations presented herein are based on the data obtained from our field explorations, at the specific locations explored on the dates indicated in the report. This report does not reflect any subsurface variations (e.g. soil types, groundwater levels, etc.) which may occur adjacent or between borings.

The nature and extent of any such variations may not become evident until construction/excavation commences. In the event such variations are encountered, Andersen Andre Consulting Engineers, Inc. may find it necessary to (1) perform additional subsurface explorations, (2) conduct in-the-field observations of encountered variations, and/or re-evaluate the conclusions and recommendations presented herein.

We at Andersen Andre Consulting Engineers, Inc. recommend that the project specifications necessitate the contractor immediately notifying Andersen Andre Consulting Engineers, Inc., the owner and the design engineer (if applicable) if subsurface conditions are encountered that are different from those presented in this report.

No claim by the contractor for any conditions differing from those expected in the plans and specifications, or presented in this report, should be allowed unless the contractor notifies the owner and Andersen Andre Consulting Engineers, Inc. of such differing site conditions. Additionally, we recommend that all foundation work and site improvements be observed by an Andersen Andre Consulting Engineers, Inc. representative.

SOIL STRATA CHANGES

Soil strata changes are indicated by a horizontal line on the soil boring profiles (boring logs) presented within this report. However, the actual strata's changes may be more gradual and indistinct. Where changes occur between soil samples, the locations of the changes must be estimated using the available information and may not be at the exact depth indicated.

SINKHOLE POTENTIAL

Unless specifically requested in writing, a subsurface exploration performed by Andersen Andre Consulting Engineers, Inc. is not intended to be an evaluation for sinkhole potential.

MISINTERPRETATION OF SUBSURFACE SOIL EXPLORATION REPORT

Andersen Andre Consulting Engineers, Inc. is responsible for the conclusions and recommendations presented herein, based upon the subsurface data obtained during this project. If others render conclusions or opinions, or make recommendations based upon the data presented in this report, those conclusions, opinions and/or recommendations are not the responsibility of Andersen Andre Consulting Engineers, Inc.

CHANGED STRUCTURE OR LOCATION

This report was prepared to assist the owner, architect and/or civil engineer in the design of the subject project. If any changes in the construction, design and/or location of the structures as discussed in this report are planned, or if any structures are included or added that are not discussed in this report, the conclusions and recommendations contained in this report may not be valid. All such changes in the project plans should be made known to Andersen Andre Consulting Engineers, Inc. for our subsequent re-evaluation.

USE OF REPORT BY BIDDERS

Bidders who are reviewing this report prior to submission of a bid are cautioned that this report was prepared to assist the owners and project designers. Bidders should coordinate their own subsurface explorations (e.g.; soil borings, test pits, etc.) for the purpose of determining any conditions that may affect construction operations. Andersen Andre Consulting Engineers, Inc. cannot be held responsible for any interpretations made using this report or the attached boring logs with regard to their adequacy in reflecting subsurface conditions which may affect construction operations.

IN-THE-FIELD OBSERVATIONS

Andersen Andre Consulting Engineers, Inc. attempts to identify subsurface conditions, including soil stratigraphy, water levels, zones of lost circulation, "hard" or "soft" drilling, subsurface obstructions, etc. However, lack of mention in the report does not preclude the presence of such conditions.

LOCATION OF BURIED OBJECTS

Users of this report are cautioned that there was no requirement for Andersen Andre Consulting Engineers, Inc. to attempt to locate any man-made, underground objects during the course of this exploration, and that no attempts to locate any such objects were performed. Andersen Andre Consulting Engineers, Inc. cannot be responsible for any buried man-made objects which are subsequently encountered during construction.

PASSAGE OF TIME

This report reflects subsurface conditions that were encountered at the time/date indicated in the report. Significant changes can occur at the site during the passage of time. The user of the report recognizes the inherent risk in using the information presented herein after a reasonable amount of time has passed. We recommend the user of the report contact Andersen Andre Consulting Engineers, Inc. with any questions or concerns regarding this issue.

Important Information about Your

Geotechnical Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

Geotechnical Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical engineering study conducted for a civil engineer may not fulfill the needs of a construction contractor or even another civil engineer. Because each geotechnical engineering study is unique, each geotechnical engineering report is unique, prepared *solely* for the client. No one except you should rely on your geotechnical engineering report without first conferring with the geotechnical engineer who prepared it. *And no one — not even you* — should apply the report for any purpose or project except the one originally contemplated.

Read the Full Report

Serious problems have occurred because those relying on a geotechnical engineering report did not read it all. Do not rely on an executive summary. Do not read selected elements only.

A Geotechnical Engineering Report Is Based on A Unique Set of Project-Specific Factors

Geotechnical engineers consider a number of unique, project-specific factors when establishing the scope of a study. Typical factors include: the client's goals, objectives, and risk management preferences; the general nature of the structure involved, its size, and configuration; the location of the structure on the site; and other planned or existing site improvements, such as access roads, parking lots, and underground utilities. Unless the geotechnical engineer who conducted the study specifically indicates otherwise, do not rely on a geotechnical engineering report that was:

- not prepared for you,
- not prepared for your project,
- not prepared for the specific site explored, or
- completed before important project changes were made.

Typical changes that can erode the reliability of an existing geotechnical engineering report include those that affect:

- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light industrial plant to a refrigerated warehouse,
- elevation, configuration, location, orientation, or weight of the proposed structure,
- composition of the design team, or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes—even minor ones—and request an assessment of their impact. *Geotechnical engineers cannot accept responsibility or liability for problems that occur because their reports do not consider developments of which they were not informed.*

Subsurface Conditions Can Change

A geotechnical engineering report is based on conditions that existed at the time the study was performed. *Do not rely on a geotechnical engineering report* whose adequacy may have been affected by: the passage of time; by man-made events, such as construction on or adjacent to the site; or by natural events, such as floods, earthquakes, or groundwater fluctuations. *Always* contact the geotechnical engineer before applying the report to determine if it is still reliable. A minor amount of additional testing or analysis could prevent major problems.

Most Geotechnical Findings Are Professional Opinions

Site exploration identifies subsurface conditions only at those points where subsurface tests are conducted or samples are taken. Geotechnical engineers review field and laboratory data and then apply their professional judgment to render an opinion about subsurface conditions throughout the site. Actual subsurface conditions may differ—sometimes significantly—from those indicated in your report. Retaining the geotechnical engineer who developed your report to provide construction observation is the most effective method of managing the risks associated with unanticipated conditions.

A Report's Recommendations Are *Not* Final

Do not overrely on the construction recommendations included in your report. *Those recommendations are not final,* because geotechnical engineers develop them principally from judgment and opinion. Geotechnical engineers can finalize their recommendations only by observing actual

subsurface conditions revealed during construction. *The geotechnical engineer who developed your report cannot assume responsibility or liability for the report's recommendations if that engineer does not perform construction observation.*

A Geotechnical Engineering Report Is Subject to Misinterpretation

Other design team members' misinterpretation of geotechnical engineering reports has resulted in costly problems. Lower that risk by having your geotechnical engineer confer with appropriate members of the design team after submitting the report. Also retain your geotechnical engineer to review pertinent elements of the design team's plans and specifications. Contractors can also misinterpret a geotechnical engineering report. Reduce that risk by having your geotechnical engineer participate in prebid and preconstruction conferences, and by providing construction observation.

Do Not Redraw the Engineer's Logs

Geotechnical engineers prepare final boring and testing logs based upon their interpretation of field logs and laboratory data. To prevent errors or omissions, the logs included in a geotechnical engineering report should *never* be redrawn for inclusion in architectural or other design drawings. Only photographic or electronic reproduction is acceptable, *but recognize that separating logs from the report can elevate risk.*

Give Contractors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can make contractors liable for unanticipated subsurface conditions by limiting what they provide for bid preparation. To help prevent costly problems, give contractors the complete geotechnical engineering report, *but* preface it with a clearly written letter of transmittal. In that letter, advise contractors that the report was not prepared for purposes of bid development and that the report's accuracy is limited; encourage them to confer with the geotechnical engineer who prepared the report (a modest fee may be required) and/or to conduct additional study to obtain the specific types of information they need or prefer. A prebid conference can also be valuable. *Be sure contractors have sufficient time* to perform additional study. Only then might you be in a position to give contractors the best information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions.

Read Responsibility Provisions Closely

Some clients, design professionals, and contractors do not recognize that geotechnical engineering is far less exact than other engineering disciplines. This lack of understanding has created unrealistic expectations that have led to disappointments, claims, and disputes. To help reduce the risk of such outcomes, geotechnical engineers commonly include a variety of explanatory provisions in their reports. Sometimes labeled "limitations" many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The equipment, techniques, and personnel used to perform a *geoenvironmental* study differ significantly from those used to perform a *geotechnical* study. For that reason, a geotechnical engineering report does not usually relate any geoenvironmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated environmental problems have led to numerous project failures.* If you have not yet obtained your own geoenvironmental information, ask your geotechnical consultant for risk management guidance. *Do not rely on an environmental report prepared for someone else.*

Obtain Professional Assistance To Deal with Mold

Diverse strategies can be applied during building design, construction, operation, and maintenance to prevent significant amounts of mold from growing on indoor surfaces. To be effective, all such strategies should be devised for the *express purpose* of mold prevention, integrated into a comprehensive plan, and executed with diligent oversight by a professional mold prevention consultant. Because just a small amount of water or moisture can lead to the development of severe mold infestations, a number of mold prevention strategies focus on keeping building surfaces dry. While groundwater, water infiltration, and similar issues may have been addressed as part of the geotechnical engineering study whose findings are conveyed in this report, the geotechnical engineer in charge of this project is not a mold prevention consultant; ***none of the services performed in connection with the geotechnical engineer's study were designed or conducted for the purpose of mold prevention. Proper implementation of the recommendations conveyed in this report will not of itself be sufficient to prevent mold from growing in or on the structure involved.***

Rely on Your ASFE-Member Geotechnical Engineer for Additional Assistance

Membership in ASFE/The Best People on Earth exposes geotechnical engineers to a wide array of risk management techniques that can be of genuine benefit for everyone involved with a construction project. Confer with your ASFE-member geotechnical engineer for more information.

ASFE THE GEOPROFESSIONAL BUSINESS ASSOCIATION

8811 Colesville Road/Suite G106, Silver Spring, MD 20910
Telephone: 301/565-2733 Facsimile: 301/589-2017
e-mail: info@asfe.org www.asfe.org

IIGER03135.0MRP

1102

lucido & associates

TRANSMITTAL
(VIA HAND DELIVERY)

Date:	April 8, 2019		
To:	Peter Walden Martin County Growth Management Dept.		
From:	Shirley Lyders		
Subject:	Mariner Village Square PUD, Revised Master Site Plan and Phase 2 Final Site Plan (Martin County Project #M160-011)	**Project No.**	18-360

Pursuant to Article 10.6.B of the Development Review Procedures, attached is the certification regarding the posting of the project sign for your records.

701 SE Ocean Blvd., Stuart, FL 34994 P (772) 220–2100 F (772) 223–0220
email: *slyders@lucidodesign.com*

1103

Doug Fitzwater
220 Hibiscus Avenue
Stuart, FL 34996

Ms. Shirley Lyders
Lucido & Associates
701 SE Ocean Blvd.
Stuart, FL 34994

Notice Proposed Development
Mariner Village Square PUD
File Number M160-011

Dear Ms. Lyders:

This is to certify that the above referenced sign was installed per Martin County requirements and comply with the standards of the notice provisions of Article 10, Section 10.6: Development Review Procedures.

Doug Fitzwater

State of Florida
County of Martin

Doug Fitzwater, who is personally known to me, acknowledged the forgoing instrument before me on April 3, 2018

Notary Public, State of Florida

SHANNON D. GARRETT
MY COMMISSION EXPIRES
May 18, 2020
#FF 951803
Bonded thru Notary Public Underwriters
NOTARY PUBLIC, STATE OF FLORIDA


NOTICE
PROPOSED DEVELOPMENT
MARINER VILLAGE SQUARE PUD
DATE, TIME AND PLACE OF HEARING(S) AND
OTHER INFORMATION AVAILABLE FROM
MARTIN COUNTY GROWTH MANAGEMENT DEPARTMENT
CALL 772-288-5495
FILE NUMBER M160-011


NOTICE
PROPOSED DEVELOPMENT
MARINER VILLAGE SQUARE PUD
DATE, TIME AND PLACE OF HEARING(S) AND
OTHER INFORMATION AVAILABLE FROM
ARTIN COUNTY GROWTH MANAGEMENT DEPARTMENT
CALL 772-288-5495
FILE NUMBER M160-011

Prepared By:
Martin County Growth Management Department
2401 S.E. Monterey Road
Stuart, FL 34996

[blank space above reserved for recording information]

BEFORE THE BOARD OF COUNTY COMMISSIONERS
MARTIN COUNTY, FLORIDA
DEVELOPMENT ORDER

RESOLUTION NUMBER _____

[REGARDING DENIAL OF REVISED MASTER AND PHASE 2 FINAL SITE PLAN APPROVAL FOR MARINER VILLAGE SQUARE PUD

WHEREAS, this Board has made the following determinations of fact:

1. Ribbin Ventures, LLC. submitted an application for a Revised PUD master and phase 2 Final site plan for the Mariner Village Square project, located on lands legally described in Exhibit A, attached hereto.
2. This Board held a public meeting on the application on February 25, 2020.
3. At the public meeting, all interested parties were given an opportunity to be heard.

NOW, THEREFORE, BE IT RESOLVED BY THE BOARD OF COUNTY COMMISSIONERS OF MARTIN COUNTY, FLORIDA, THAT:

A. The request for a Revised master and phase 2 Final Site Plan for the Mariner Village Square PUD project is denied because XXXXX.

B. This resolution shall be recorded in the public records of Martin County. A copy of this resolution shall be forwarded to the applicant(s) by the Growth Management Department subsequent to recording.

DULY PASSED AND ADOPTED THIS 25TH DAY OF FEBRUARY, 2020.

ATTEST:

BOARD OF COUNTY COMMISSIONERS
MARTIN COUNTY, FLORIDA

BY:_____
CAROLYN TIMMANN
CLERK OF THE CIRCUIT COURT
AND COMPTROLLER

BY: _____
HAROLD E. JENKINS, CHAIRMAN

1105

APPROVED AS TO FORM AND LEGAL SUFFICIENCY:

BY: _____
KRISTA A. STOREY
SENIOR ASSISTANT COUNTY ATTORNEY

ATTACHMENTS:

Exhibit A, Legal Description

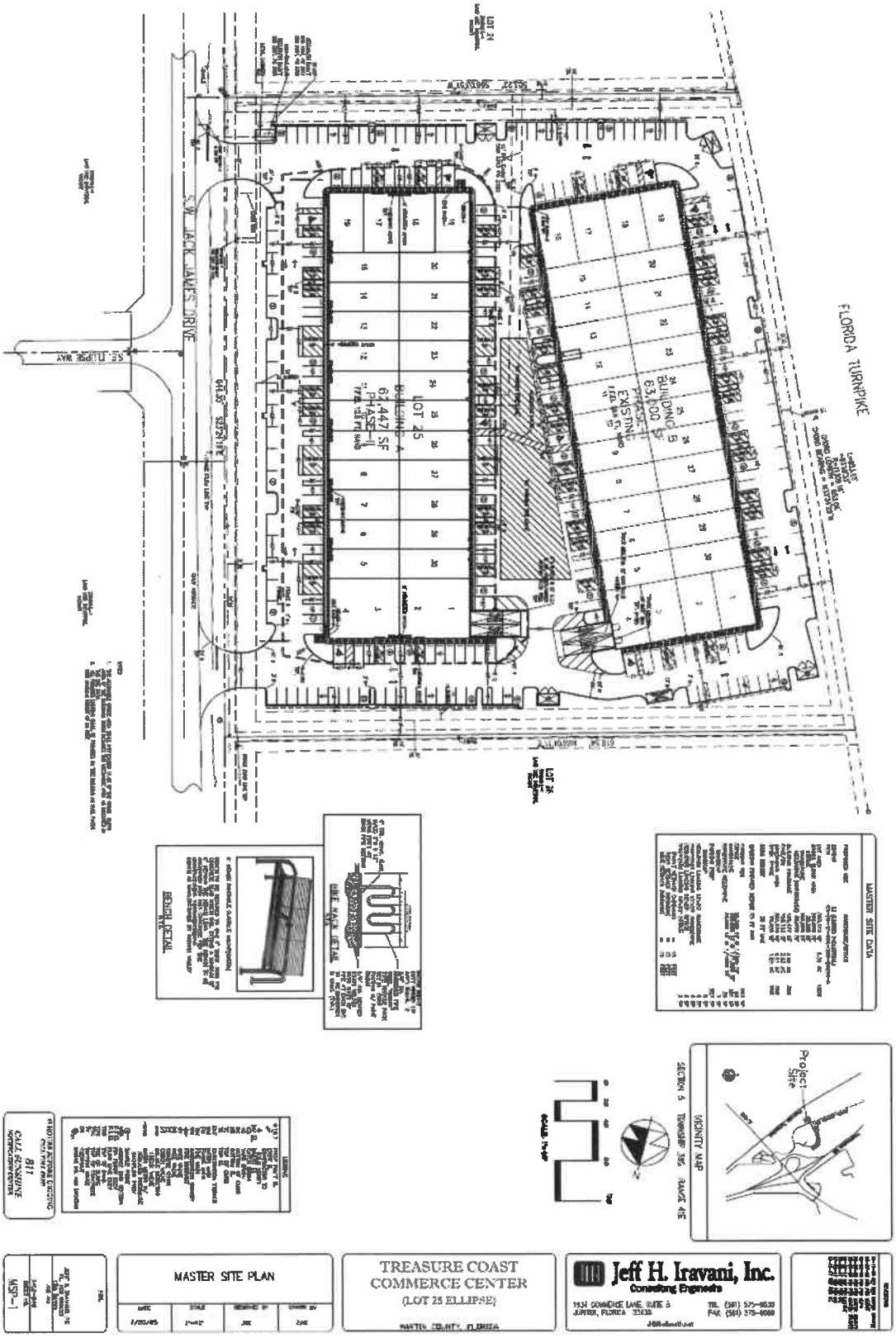
Exhibit "A"
Legal Description

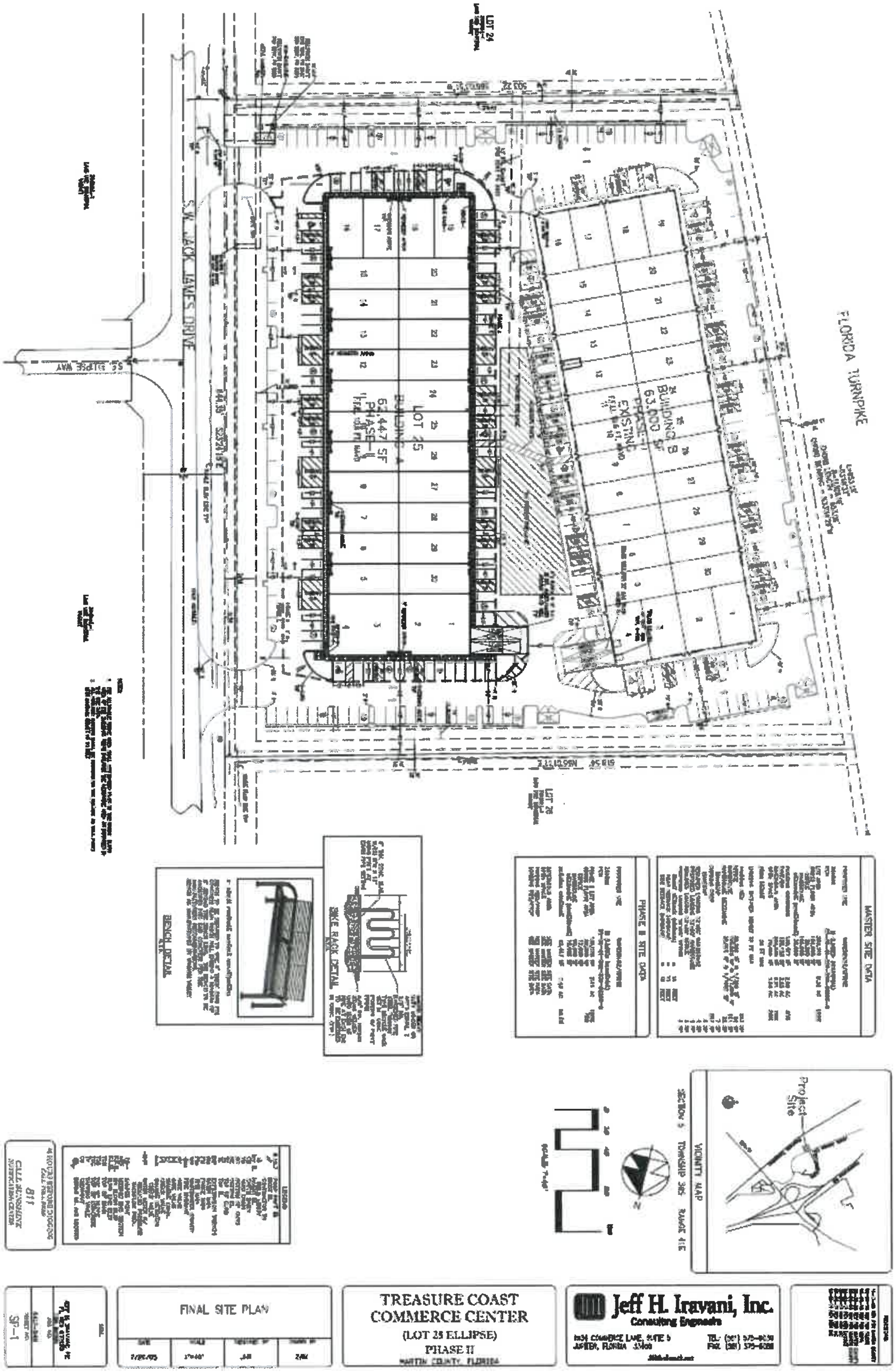
Lot 25 of the Plat of THE ELLIPSE, according to the Plat thereof as recorded in Plat Book 11 at Page 84 of the Public Records of Martin County, Florida.

Total Acreage: 8.34

PCN: 05-39-41-002-000-00250-6

Exhibit "B"


SW JACK JAMES DRIVE
FLORIDA TURNPIKE
LOT 25
BUILDING 5
63,000 SF
PHASE I
EXISTING
BUILDING 4
61,447 SF
PHASE I
MASTER SITE DATA
BENCH DETAIL
BIKE RACK DETAIL
VICINITY MAP
Project Site
SECTION 5 TOWNSHIP 39S RANGE 41E
MASTER SITE PLAN
TREASURE COAST
COMMERCE CENTER
(LOT 25 ELLIPSE)
Jeff H. Iravani, Inc.
Consulting Engineers


FLORIDA TURNPIKE
S.W. JACK JAMES DRIVE
S.E. ELLIPSE WAY
LOT 25
BUILDING A
PHASE II
BUILDING B
EXISTING
MASTER SITE DATA
PHASE II SITE DATA
BIKE RACK DETAIL
BENCH DETAIL
VICINITY MAP
Project Site
FINAL SITE PLAN
TREASURE COAST
COMMERCE CENTER
(LOT 25 ELLIPSE)
PHASE II
MARTIN COUNTY, FLORIDA
Jeff H. Iravani, Inc.
Consulting Engineers
SP-1

1110

DPQJ-1

Peter W. Walden
Principal Planner
Martin County Growth Management Department
pwalden@martin.fl.us Office772-219-4923
2401 SE Monterey Road Stuart, FL 34996

COUNTY EXHIBIT # 2

Experience

Principal Planner, Martin County, FL 2018- present
- Project Coordinator- development application and land development regulation review
- Project Coordinator for all County projects for development review.
- Manage and process all zoning variances.
- Provide assistance with permitting and zoning applications.
- Draft Land Development Regulation amendments.

Senior Planner, Martin County, Fl. 2015- 2018
- **Development Review**: Project coordinator for development and zoning applications.
- Provide review of development applications for consistency with the Comprehensive Growth Management Plan and the Land Development Regulations.

Development Compliance Planner, City of Palm Beach Gardens, Palm Beach Gardens, Fl. 2014- 2015
- **Development Review:** Review development and permit applications for compliance with land development code. Monitor development construction for compliance with development orders and environmental compliance. Provide related documents; draft time extensions, build out determinations, administrative amendments.

Zoning Compliance, Village of North Palm Beach, NPB, Fl. 2012- 2014
- **Plan Review**: Member of the DRC, participate in all development review, focus on zoning regulations and land development policy and compliance. Review building permits for code compliance. Prepare and present projects to the Planning Commission, and maintain all corresponding files.

Sales Associate, The Home Depot, Jupiter, FL 2010-2012
- Worked as a Sales Associate while attending FAU.

Landscape Design Manager, Ginn Company, Celebration, FL. 2004-2008
- Part of a development team responsible for managing the construction and maintenance of Resort Communities.
- Projects included: golf courses, land development and earthwork, utilities and irrigation, streetscapes, mitigation projects and wetland construction, parks, Clubhouses, and PGA Tour events in the southeast and the Bahamas.
- Worked with project managers and consultants on development compliance with SFWMD, DEP, Army Corp of Engineers, and local municipalities.

Education & Certifications

Florida Atlantic University, Boca Raton, FL (GPA 3.8)
B.P.M. **Bachelor of Public Management** (Administration), minor in Geography, May 2012
Course work in; Urban Planning, GIS, Emergency Management, Program Evaluation, Transportation

Indian River State College, Stuart, FL (GPA: 3.75)
A.A, **Environmental Science**, May 2010

Government Internship, Town of Jupiter, Fl. May-August 2011 Planning and Zoning, Business Development
Member of the American Planning Association

FILED FOR RECORD
COMMISSION RECORDS
MARTIN COUNTY, FL
Date 02/25/2020 Time
CAROLYN TIMMANN
CLERK OF CIRCUIT COURT
By MKV D.C.